

Slope stability slab foundation

Chapter 4 (FEM-program) contains features:

1. Compact complete **Input Receipt** $\Rightarrow$ Less than 10 pages.
2. **Phi-c Reduction performs safety factor analysis for nonlinear material.**
 $\Rightarrow$ Safe factor is presented including associated deformations.
3. **Branch Analysis** enables performing several independent analyses within in same model.
4. **Material Modified Mohr-Coulomb (MMC) defines plastic soil material.**
 $\Rightarrow$ Useful when defining concrete structures.

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Appendix

1. Input receipt (FEM-program)

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1. GENERAL

A slab foundation is evaluated using three different methods:

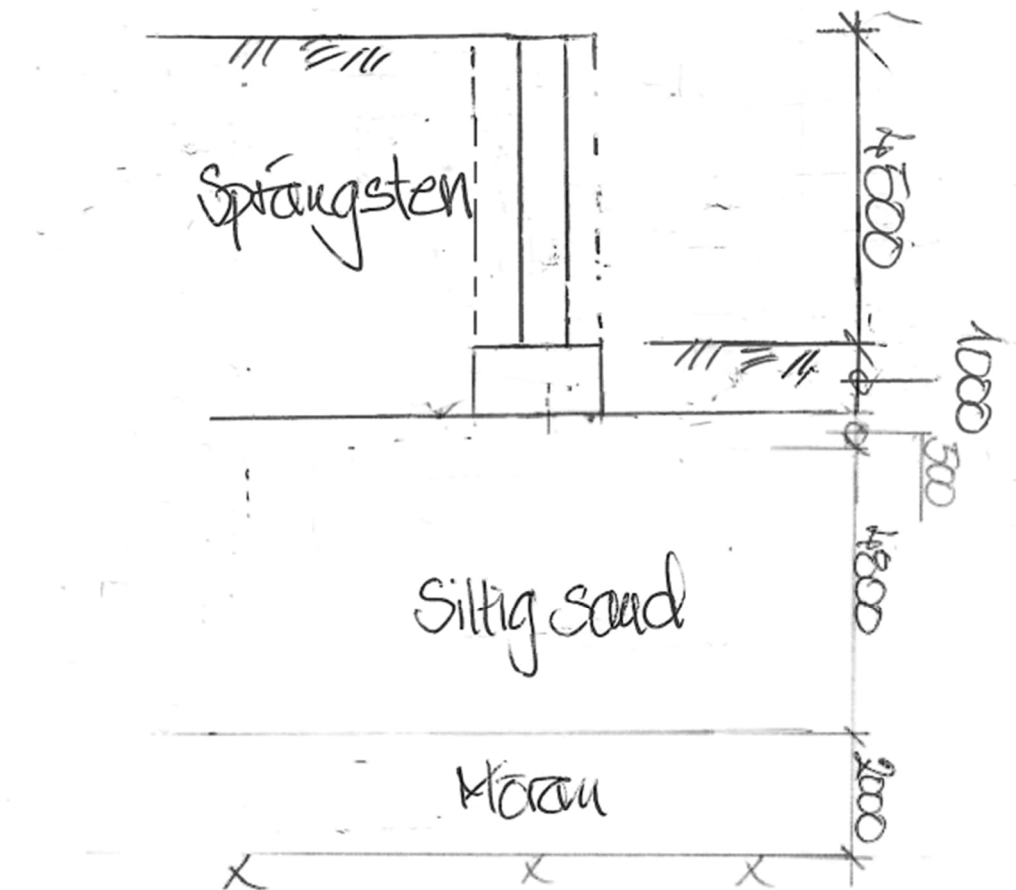
- General bearing capacity equation ("Allmänna bärrighetsekvationen")
- Circular slipe surface using SLOPE/W using ultimate limit safety.
- Slope stability analysis using FEM-program.

Summary of safety factors (SF):

Method	SF	Page
General bearing capacity equation	0.56	7
Circular slipe surface	1.17	7
Slope stability analysis	1.19	23

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1.1 GEOTECHNICAL CROSSECTION



ELEVATION

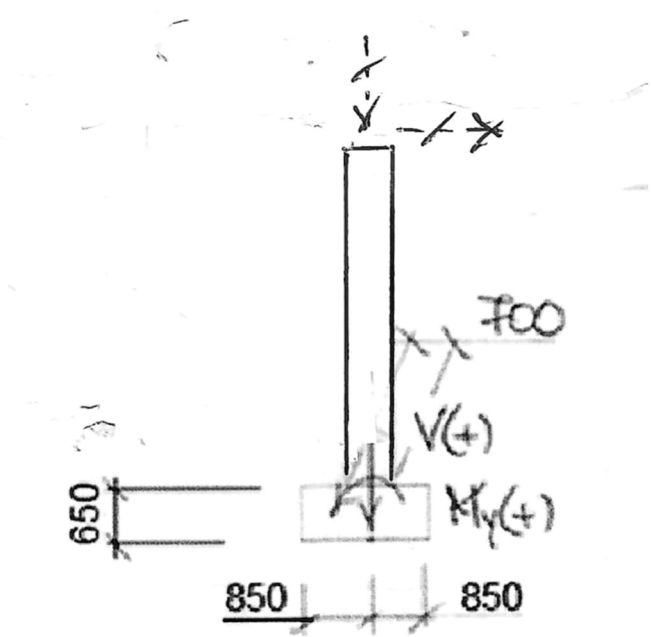
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Geotechnical parameters:

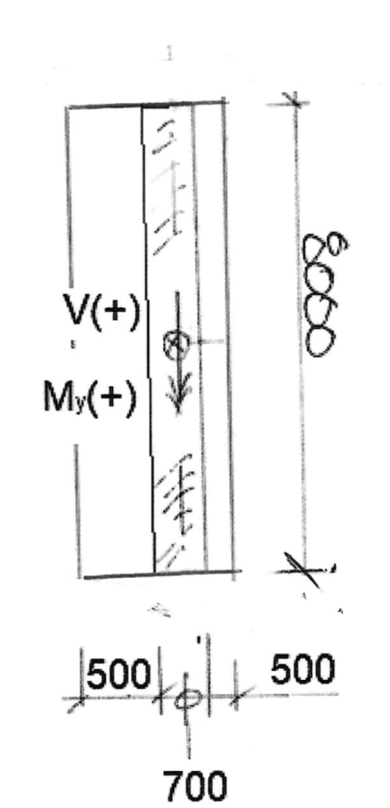
Material	Level	ρ	ρ'	ϕ_k	E_k
"Sprängsten"	20.3-19.8	18	11	45	50
"Siltig sand"	19.8-15.0	20	11	38	35
"Morän"	15.0-13.0	20	11	39	45
"Berg"	13.0	-	-	-	-
-	m	kN/m ³	kN/m ³	deg	MPa

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1.2 GEOMETRY



ELEVATION



PLAN

Bottom slab

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1.3 DESIGN LOAD EFFECT

$$V_d = 3566kN$$

$$H_{xd} = 1570kN$$

$$H_{yd} = 39kN$$

$$M_{xd} = 386kNm$$

$$M_{yd} = 21kNm$$

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2. GEOTECHNICAL BEARING CAPACITY EQUATION

General bearing capacity equation is determined using SS-EN 1997-1 annex D & TRVINFRA-002300 section 9.2.1.2.1 (“Allmänna bärrighetsekvationen”), see other documentation.

3. CIRCULAR SLIPE SURFACE

Circular slipe surface safety is determined using SLOPE/W for ultimate limit (ULS) see other documentation.

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4. SLOPE STABILITY ANALYSIS

4.1 GENERAL

Soil & abutment are modelled using 2D triangular quadric plain strain elements (TNP6).

FEM-program has function *Phi-c Reduction* performs a safety factor analysis on nonlinear material.

Plastic load model *Modified Mohr-Coulomb* (MMC) to define both soil and concrete.

FEM-program has function *Branch Analysis* that enables performing several analyses within same model. This capability is used to perform a safety factor analysis on nonlinear material.

To model concrete abutment a fictious soil material is chosen. A cohesion material with very high cohesion (2 MPa) is used.

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4.2 SYSTEM ANALYSIS

4.2.1 Safety factor analysis

Safety factor analysis is performed using function *Phi-c Reduction* with starting values seen below.

Phi-c Reduction

	Value
Starting value for safety factor	1,1
Minimum change in safety factor	0,01
Maximum number of steps to use	40

Name
Phi-c1
(1)

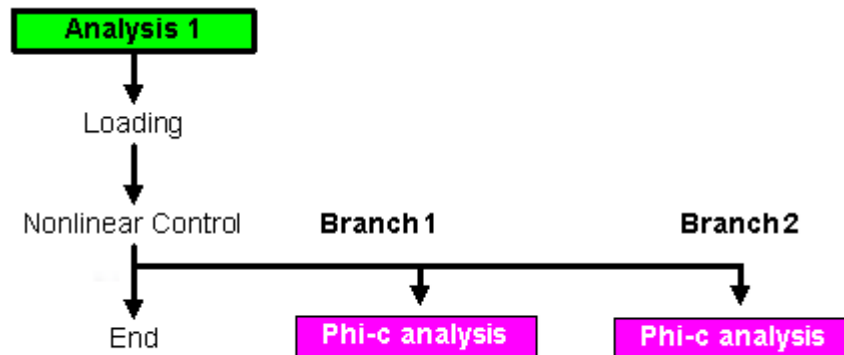
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4.2.3 Branch Analysis

Branch Analysis seen below is used within same model.

Branch 1: Soil

Branch 2: Soil + Surface load



Schematic sketch

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4.2.4 Nonlinear analysis

Nonlinear analysis is performed using increments using Newton_Raphson numerical procedure. These *Incrementation* as chosen to be performed *Manual* seen convergence generally more stable, see chosen parameters in dialogue box.

Nonlinear & Transient

Incrementation

☒ Nonlinear

Incrementation:

Starting load factor:

Max change in load factor:

Max total load factor:

☒ Adjust load based on convergence

Iterations per increment:

☐ Time domain

Initial time step:

Total response time:

☐ Automatic time stepping

Solution strategy

☐ Same as previous loadcase

Max number of iterations:

Residual force norm:

Displacement norm:

Incremental LUSAS file output

☐ Same as previous loadcase

Output file:

Plot file:

Restart file:

Max number of saved restarts:

Log file:

History file:

Max number of iterations:

For each load increment if convergence is not achieved.

Residual force norm:

The limit for the sum of the squares of all residual forces as a percentage of the sum of the squares of all external forces, including reactions (rdnm). By default only translation degrees of freedom are considered. So for a value of 0.1 the convergence of the solution at each load increment will be achieved when the out of balance forces are as less than 0.1% of the reactions.

Displacement norm:

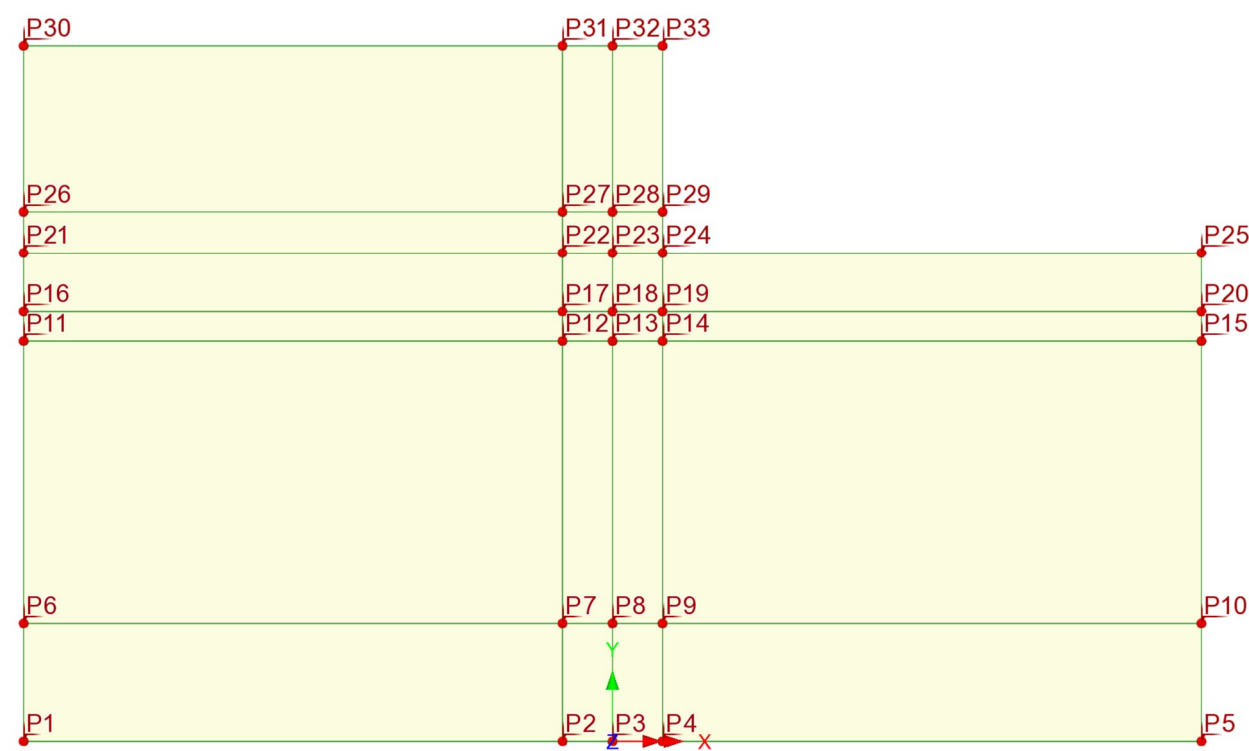
The limit for the sum of the squares of the iterative displacements as a percentage of the sum of the squares of the incremental displacements (DTNRM). Only translation degrees of freedom are considered

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4.3 STATIC SYSTEM

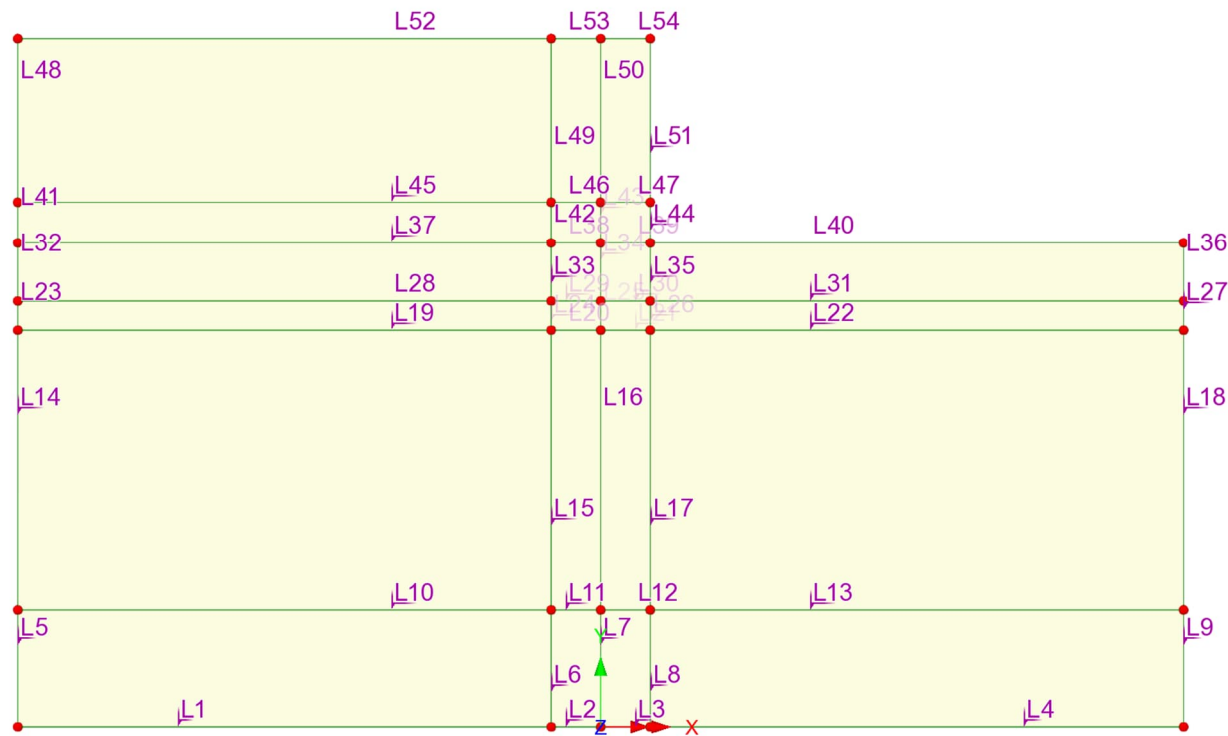
4.3.1 Geometry

Points:

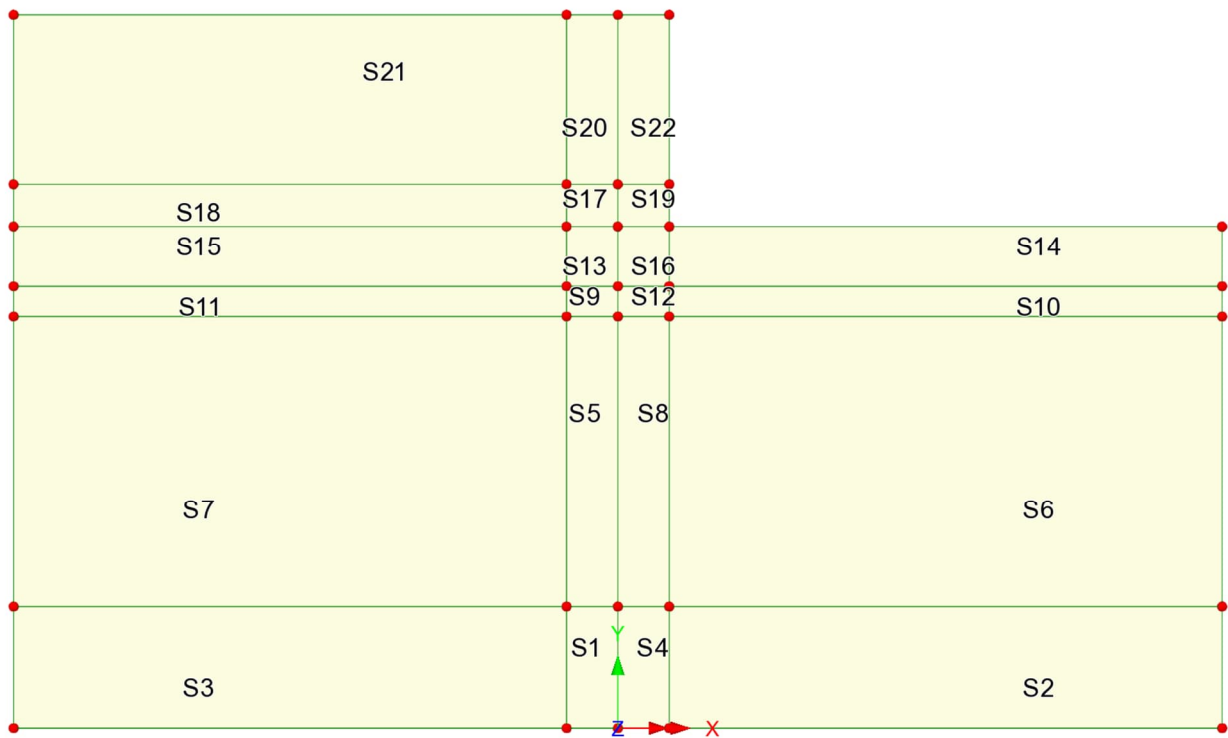


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Lines:

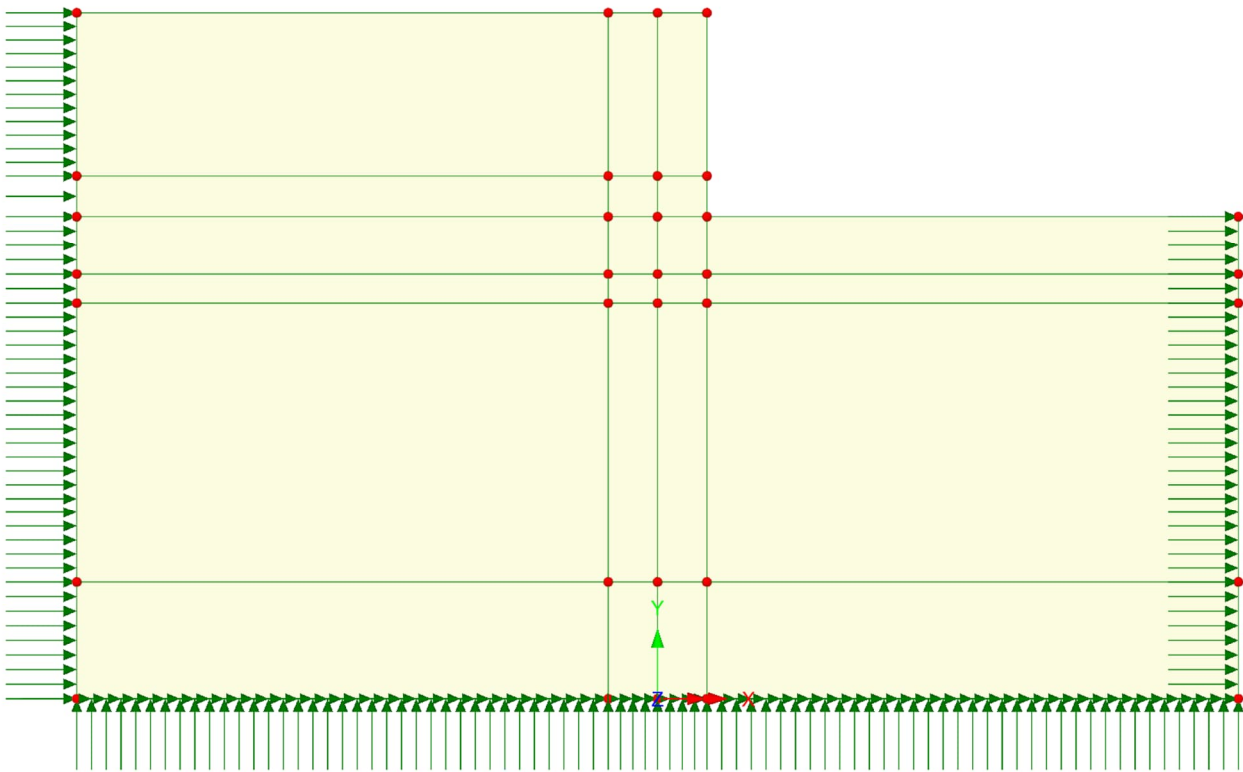


Surface:



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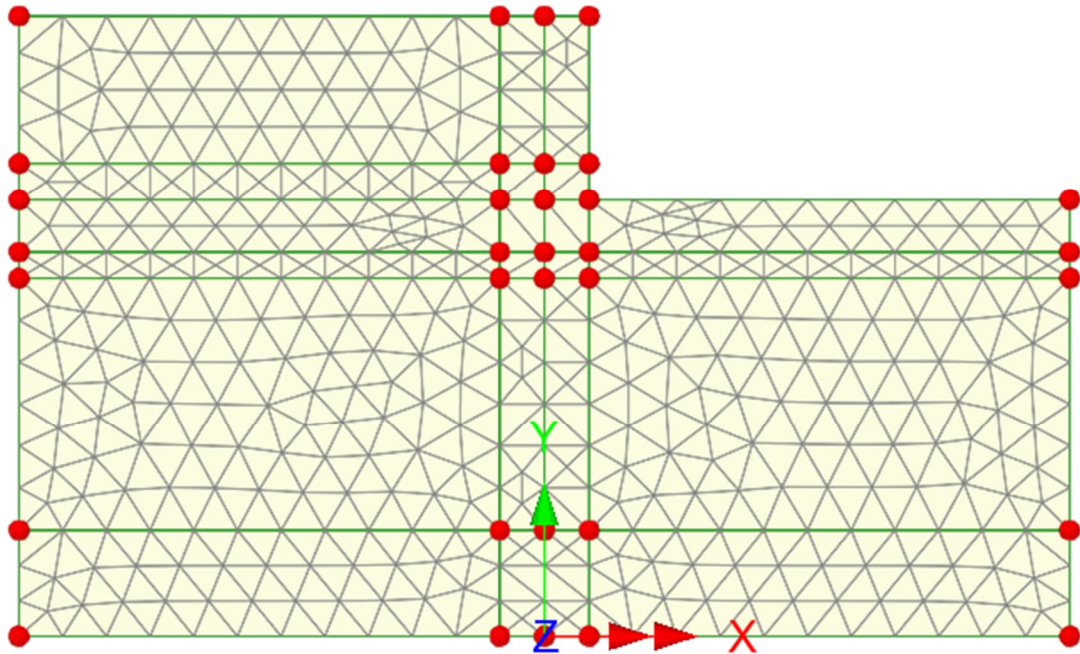
4.3.2 Boundary



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4.3.3 MESH: Beam element (BMI2)

Soil & foundation are modelled as seen below.

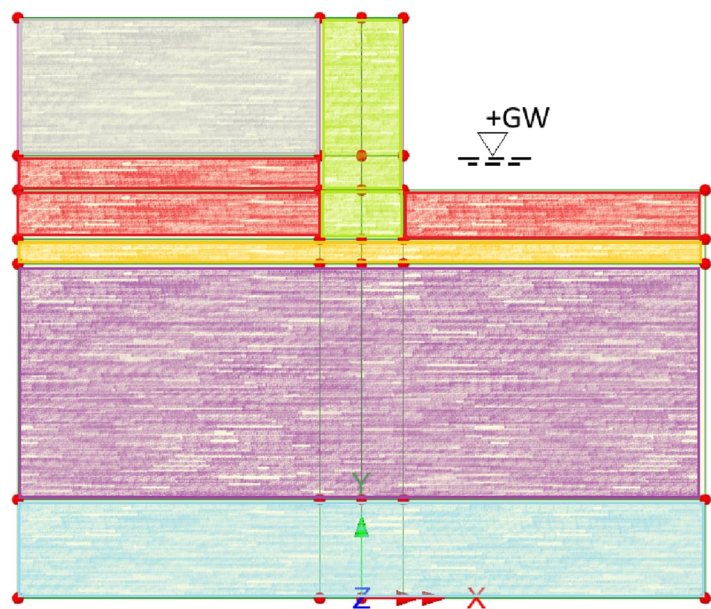


Name	Element size	Irregular mesh
TNP6	0.8 m	Yes

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4.3.4 Material

All materials are isotropic plastic material using *Modified Mohr-Coulomb* as seen below.



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Elastic properties:

Material	Below GW	Mass density	ν	E-modulus
Concrete	-	0.5 ^{1.)}	0,3	34000
"Motfyllning"	No	2.0	0,3	50
"Motfyllning - HW"	Yes	1.1	0,3	50
"Packad fyllning - HW"	Yes	1.1	0,3	50
"Siltig sand - HW"	Yes	1.1	0,3	35
"Moran - HW"	Yes	1.1	0,3	45
-	-	ton/m ³	-	MPa

1.) Fictitious density.

Plastic properties (incl. K_0 initialisation):

Material	Plastic modell	ϕ_{friktion}	ϕ_{dilation}	c_u	$\sigma_{t, \text{cut-off}}$	$\sigma_{c, \text{cut-off}}$	K_0
Concrete	Modified Mohr Coulomb ^{2.)}	0	0	$2 \cdot 10^3$	-	-	1.0 ^{4.)}
"Motfyllning"	Modified Mohr Coulomb ^{3.)}	38	38	0	0	109	1.0 ^{4.)}
"Motfyllning - HW"	Modified Mohr Coulomb ^{3.)}	38	38	0	0	109	1.0 ^{4.)}
"Packad fyllning - HW"	Modified Mohr Coulomb ^{3.)}	38	38	0	0	109	1.0 ^{4.)}
"Siltig sand – HW"	Modified Mohr Coulomb ^{3.)}	31	31	0	0	109	1.0 ^{4.)}
"Moran - HW"	Modified Mohr Coulomb ^{3.)}	32	32	0	0	0	1.0 ^{4.)}
-	-	deg	deg	kPa	kPa	kPa	-

2.) No cut-off due to compression or tension stress.

3.) Rankine using cut-off stress at $\sigma_{t, \text{cut-off}}$ and $\sigma_{c, \text{cut-off}}$

4.) When non-linear analysis starts, initially K_0 is applied.
($K_0 = 1.0 \rightarrow \sigma_h = \sigma_v$ which is considered on safe side).

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4.4 LOADS

4.4.1 Soil weight (Gravity)

Soil weight is determined material density.

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4.4.2 Surfaced load

Surface load is defined as a global distributed load.

The vertical load is reduced due fictious weight of concrete.

$$V_d = 3566kN \quad : \text{ see page 6}$$

$$H_{xd} = 1570kN \quad : \text{ see page 6}$$

$$H_{x.earth} = 515kN \quad : \text{ see page 10}$$

$$B = 1.70m \quad : \text{ see page 5}$$

$$L_{ef} = 8.28m \quad : \text{ see page 8}$$

$$H_{xd.red} = H_{xd} - \psi \gamma_{earth} \cdot H_{x.earth} = 1570kN - 1.45 \cdot 515kN = 823kN$$

$$\rightarrow P_x = \frac{H_{xd.red}}{B \cdot L_{ef}} = \frac{823kN}{1.70m \cdot 8.28m} = -60 \frac{kN}{m^2}$$

$$V_{d.red} = V_d - B \cdot H = 3566kN - 1.70m \cdot 4.5m \cdot 5 \frac{kN}{m^3} = 3528kN$$

$$\rightarrow P_y = -\frac{V_{d.red}}{B \cdot L_{ef}} = -\frac{3528kN}{1.70m \cdot 8.28m} = -250 \frac{kN}{m^2}$$

$$P_d = \sqrt{P_x^2 + P_y^2} = 257 \frac{kN}{m^2}$$

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Global Distributed ✕

Analysis category

2D Inplane

☐ Total

☒ Per unit length

☐ Per unit area

Component	Value
X Direction	60,0
Y Direction	-250,0

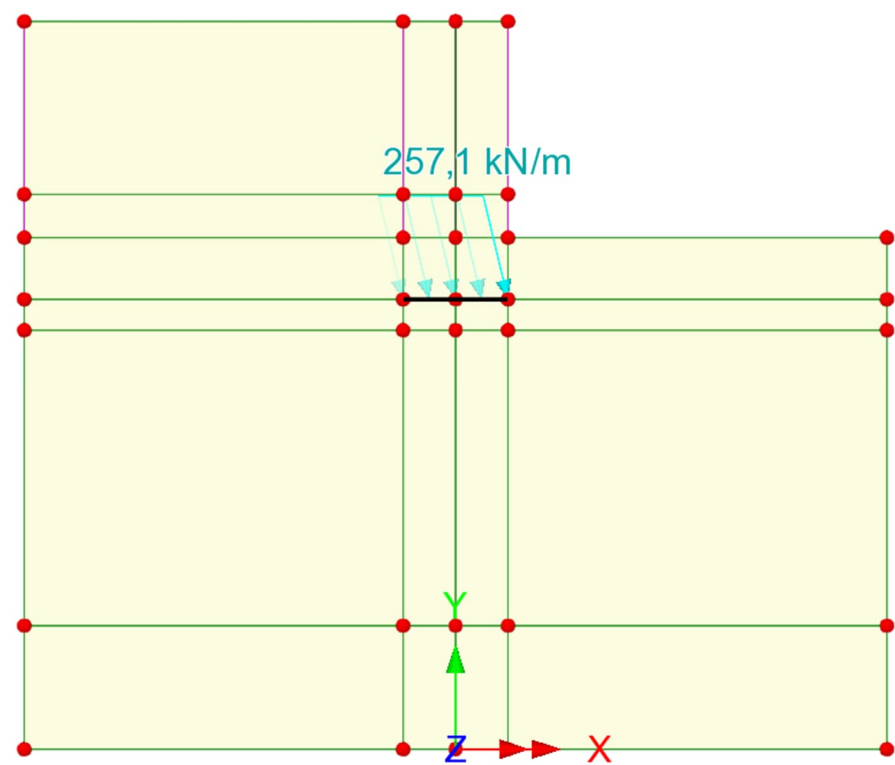
Name

Surface loading

▲▼

(1)

$$P_d = \sqrt{Px^2 + Py^2} = 257 \frac{kN}{m^2}$$



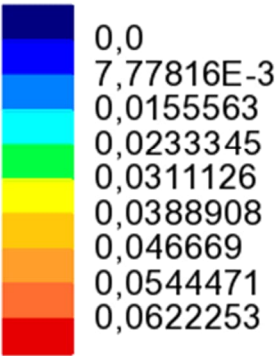
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4.4 RESULTS (FEM-program)

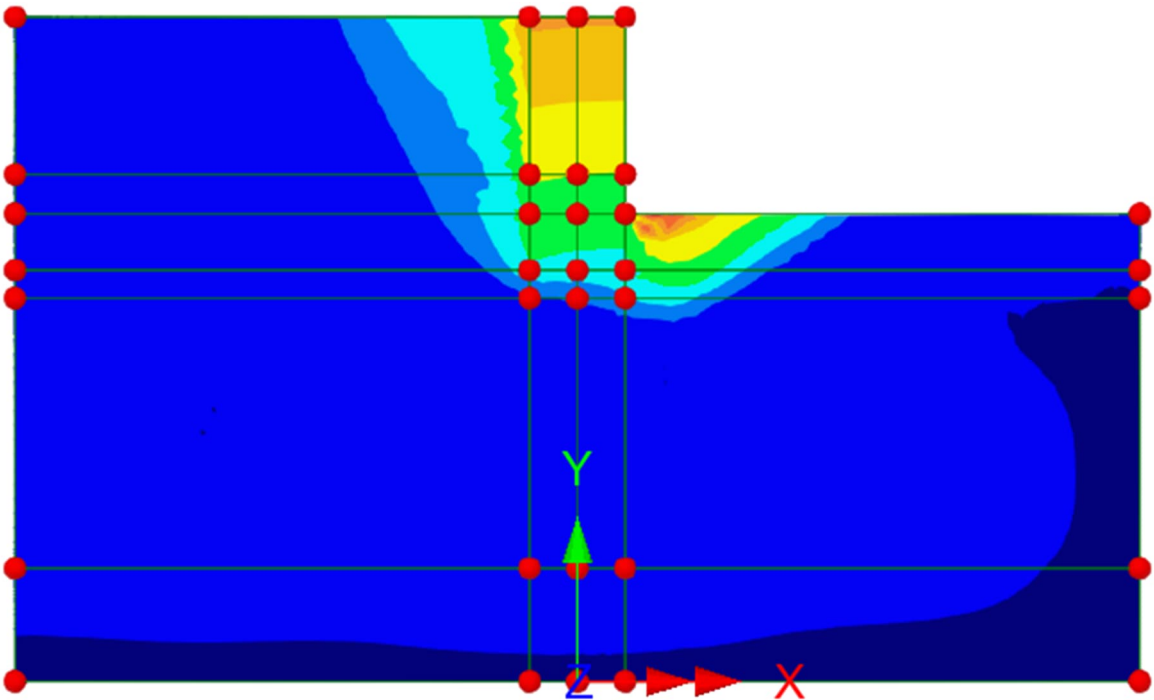
4.4.1 Branch 1: Soil

→ Safety factor FS = 1.98

Analysis: Branch 1: Soil
Loadcase: 2:Loadcase 2, 12:Increment 12 SF=1.976
Results file: s8_rev2~Branch 1_ Soil.mys
Entity: Displacement
Component (Nodal): RSLT (Units: m)



Maximum 0,0700035 at node 1261
Minimum 0,0 at node 1



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Scale: 1: 166,573

Zoom: 56,7427

Eye: (0,0; 0,0; 1,0)

Nonlinear analysis

Analysis: Branch 1: Soil

Loadcase: 2:Loadcase 2, 12:Increment 12 SF=1.976

Results file: s8_rev2~Branch 1_ Soil.mys

Maximum displacement 0,0700035 at node 1261

Deformation exaggeration: 14,2769

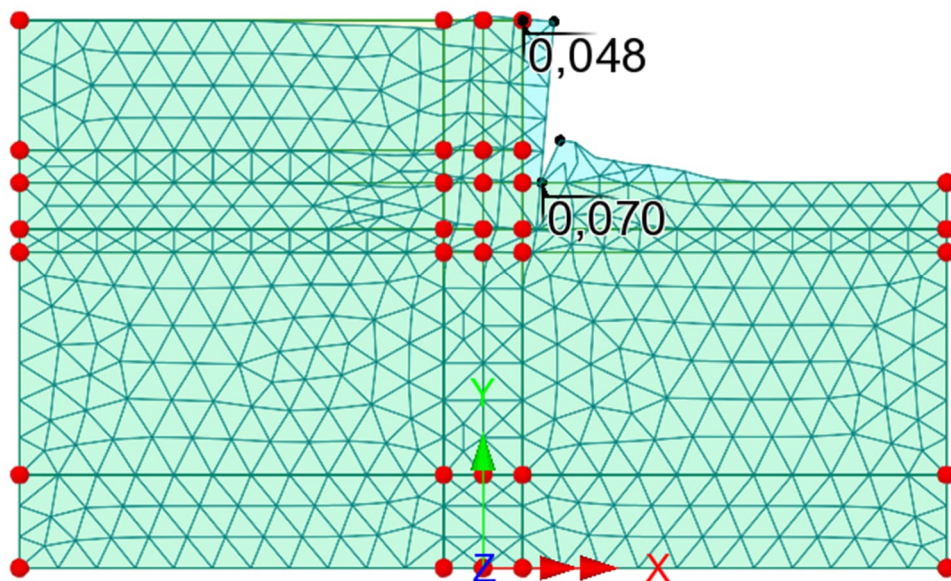
Peak/value entity: Displacement

Peak/value component: RSLT (Units: m)

Peak range(%): 100,0

Peak/value maximum 0,0700035 at node 1261

Peak/value minimum 0,0 at node 1



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4.4.2 Branch 2: Soil + Surface load

→ Safety factor $FS = 1.19$

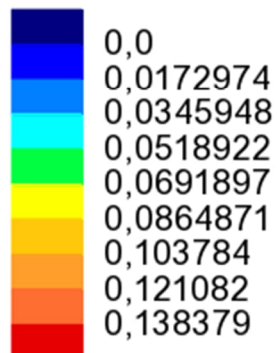
Analysis: Branch 2: Soil + surface load

Loadcase: 3:Loadcase 3, 14:Increment 14 $SF=1.185$

Results file: s8_rev2~Branch 2_ Soil + surface load.mys

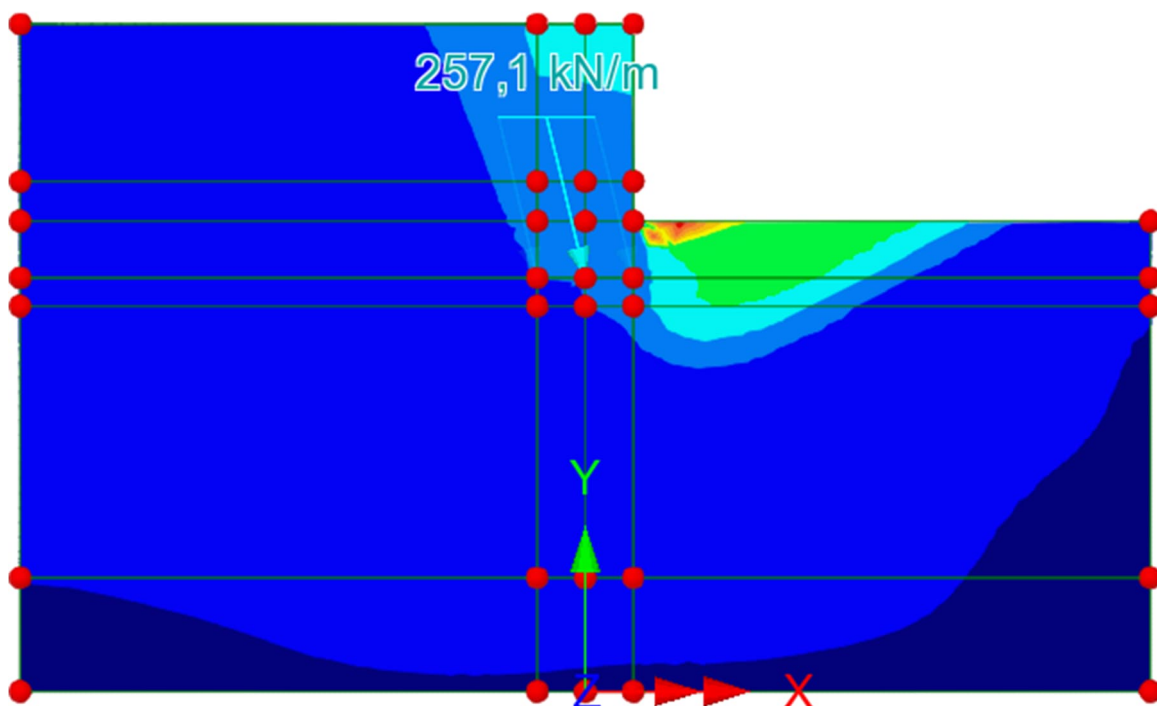
Entity: Displacement

Component (Nodal): RSLT (Units: m)



Maximum 0,155677 at node 1260

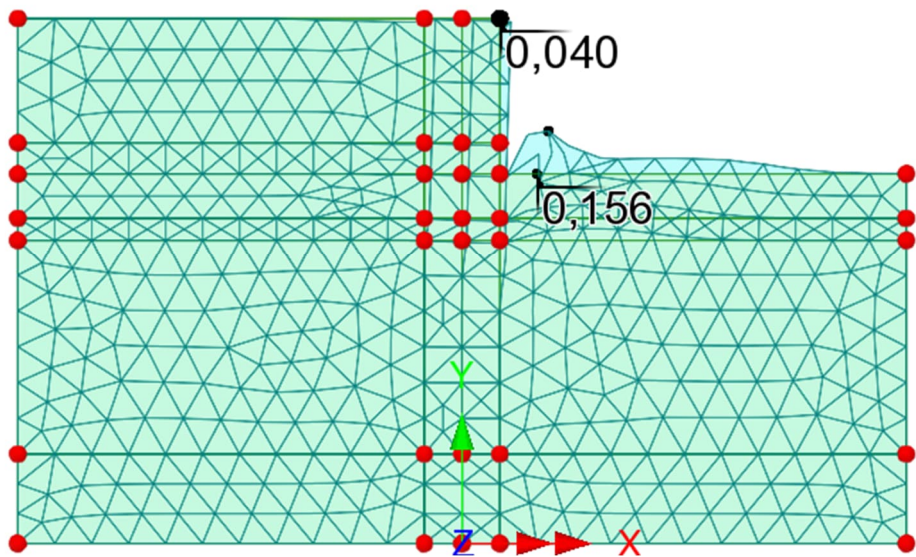
Minimum 0,0 at node 1



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Scale: 1: 166,573
 Zoom: 56,7427
 Eye: (0,0; 0,0; 1,0)
 Nonlinear analysis
 Analysis: Branch 2: Soil + surface load
 Loadcase: 3:Loadcase 3, 14:Increment 14 SF=1.185
 Results file: s8_rev2~Branch 2_ Soil + surface load.mys
 Maximum displacement 0,155677 at node 1260
 Deformation exaggeration: 6,41994

 Peak/value entity: Displacement
 Peak/value component: RSLT (Units: m)
 Peak range(%): 100,0
 Peak/value maximum 0,155677 at node 1260
 Peak/value minimum 0,0 at node 1



Title: Input receipt

Model Units: kN,m,t,s,C
Report Units: kN,m,t,s,C

Model Title:
Model File: s8

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- 10. Loading**
- 11. Loadcase**

1. Phi-c reduction

Attribute: 1 Title: Ph-c1

Sub Type = Phi-c reduction

Starting value: 1.1
Minimum change in safety factor: 0.01
Maximum number of steps in use: 60

2. Branch

Attribute: 1

Title: Branch 1: Soil

Sub Type = Branch

Analysis: Analysis 1

Attribute: 2

Title: Branch 2: Soil + surface load

Sub Type = Branch

Analysis: Analysis 1

3. Analysis

Analysis	Type	Branch	Phi-c reduction
1	Nonlinear & Transient	None	None
1	Nonlinear & Transient	1	Ph-c1
1	Nonlinear & Transient	2	Ph-c1

4. Points

Point	X coordinate	Y coordinate	Z coordinate
1	-10,0	0,0	0,0
2	-0,9	0,0	0,0
3	0,0	0,0	0,0
4	0,9	0,0	0,0
5	10,0	0,0	0,0
6	-10,0	2,0	0,0
7	-0,9	2,0	0,0
8	0,0	2,0	0,0
9	0,9	2,0	0,0
10	10,0	2,0	0,0
11	-10,0	6,8	0,0
12	-0,9	6,8	0,0
13	0,0	6,8	0,0
14	0,9	6,8	0,0
15	10,0	6,8	0,0
16	-10,0	7,3	0,0
17	-0,9	7,3	0,0
18	0,0	7,3	0,0
19	0,9	7,3	0,0
20	10,0	7,3	0,0
21	-10,0	8,3	0,0
22	-0,9	8,3	0,0
23	0,0	8,3	0,0
24	0,9	8,3	0,0
25	10,0	8,3	0,0
26	-10,0	9,0	0,0
27	-0,9	9,0	0,0
28	0,0	9,0	0,0
29	0,9	9,0	0,0
30	-10,0	11,8	0,0
31	-0,9	11,8	0,0
32	0,0	11,8	0,0
33	0,9	11,8	0,0

5. Lines

Line	Points	Line	Points
1	2;1	2	3;2
3	3;4	4	4;5
5	1;6	6	7;2
7	3;8	8	9;4
9	5;10	10	6;7
11	8;7	12	8;9
13	10;9	14	6;11
15	12;7	16	8;13
17	14;9	18	10;15
19	11;12	20	13;12
21	13;14	22	15;14
23	11;16	24	17;12
25	13;18	26	19;14
27	15;20	28	16;17
29	18;17	30	18;19
31	20;19	32	16;21
33	22;17	34	18;23
35	24;19	36	20;25
37	21;22	38	23;22
39	23;24	40	25;24
41	21;26	42	27;22
43	23;28	44	29;24
45	26;27	46	28;27
47	28;29	48	26;30
49	31;27	50	28;32
51	33;29	52	30;31
53	32;31	54	32;33

6. Surfaces

Surface	Lines	Surface	Lines
1	7;11;6;2	2	9;13;8;4
3	5;1;6;10	4	7;3;8;12
5	16;20;15;11	6	18;22;17;13
7	14;10;15;19	8	16;12;17;21
9	25;29;24;20	10	27;31;26;22
11	23;19;24;28	12	25;21;26;30
13	34;38;33;29	14	36;40;35;31
15	32;28;33;37	16	34;30;35;39
17	43;46;42;38	18	41;37;42;45
19	43;39;44;47	20	50;53;49;46
21	48;45;49;52	22	50;47;51;54

7. Mesh

Attribute: 1 Title: TPN6 1,0 m
Sub Type = Surface Mesh Element Type = TPN6

Property

Element size
Quadrilateral element angle limit
Size multiplier
edge deviation
Element defined by name
Single feature joint

Symbol	Value
size	0,8
quadAngle	30,0
sizeMultiplier	1,1
edgeDeviation	0,0
DefinedByName	false
isSingleFtrJnt	false

Assignment to Surfaces:
1T22

8. Material

Attribute: 2 Title: Motfylling
Sub Type = Isotropic Material
Assigned in: Analysis 1

Property

Young's modulus
Poisson's ratio
Density
Type
Friction angle
Dilation angle
Damping factor
Single cohesion value
Single tensile value
Single compressive value
Type
Model
Calculate initial void ratio
Young's modulus for Ko
Coefficient of lateral earth pressure

Symbol

E
nu
rho
type_8
fric
dil
dmpFct
CSngle
QmaxSngle
QminSngle
type_6
model
calcVoid
Eko
Ko

Value

50000,0
0,3
2,0
"Rankine"
38,0
38,0
0,0
0,0
0,0
1000000000,0
"elastic"
"Elastic"
false
0,0
1,0

~~**Loadcase ID: 1 Title: Loadcase 1**~~ (Information "Assigned in" seems sufficient)

Assignment to Surfaces:

21

Attribute: 3 Title: Packad fyllning - HW
Sub Type = Isotropic Material
Assigned in: Analysis 1

Property

Young's modulus
Poisson's ratio
Density
Type
Friction angle
Dilation angle
Damping factor
Single cohesion value
Single tensile value
Single compressive value
Type
Model
Calculate initial void ratio
Young's modulus for Ko
Coefficient of lateral earth pressure

Symbol

E
nu
rho
type_8
fric
dil
dmpFct
CSngle
QmaxSngle
QminSngle
type_6
model
calcVoid
Eko
Ko

Value

50000,0
0,3
1,1
"Rankine"
38,0
38,0
0,0
0,0
0,0
1000000000,0
"elastic"
"Elastic"
false
0,0
1,0

~~**Loadcase ID: 1 Title: Loadcase 1**~~

Assignment to Surfaces:

9T12

Attribute: 4 Title: Motfylling- HW

Sub Type = Isotropic Material

Assigned in: Analysis 1

Property

Young's modulus
Poisson's ratio
Density
Type
Friction angle
Dilation angle
Damping factor
Single cohesion value
Single tensile value
Single compressive value
Type
Model
Calculate initial void ratio
Young's modulus for Ko
Coefficient of lateral earth pressure

Symbol

E
nu
rho
type_8
frc
dil
dmpFct
CSngle
QmaxSngle
QminSngle
type_6
model
calcVoid
Eko
Ko

Value

50000,0
0,3
1,1
"Rankine"
38,0
38,0
0,0
0,0
0,0
1000000000,0
"elastic"
"Elastic"
false
0,0
0,5

~~**Loadcase ID: 1 Title: Loadcase 1**~~

Assignment to Surfaces:

14;15;18

Attribute: 6 Title: Concrete

Sub Type = Isotropic Material

Assigned in: Analysis 1

Property

Young's modulus
Poisson's ratio
Density
Type
Friction angle
Dilation angle
Damping factor
Single cohesion value
Single tensile value
Single compressive value
Type
Model
Calculate initial void ratio
Young's modulus for Ko
Coefficient of lateral earth pressure

Symbol

E
nu
rho
type_8
frc
dil
dmpFct
CSngle
QmaxSngle
QminSngle
type_6
model
calcVoid
Eko
Ko

Value

34000000,0
0,3
0,5
"No cut-off"
0,0
0,0
0,0
2000,0
0,0
0,0
"elastic"
"Elastic"
false
0,0
1,0

~~**Loadcase ID: 1 Title: Loadcase 1**~~

Assignment to Surfaces:

13;16;17;19;20;22

Attribute: 7 Title: Siltig sand - HW

Sub Type = Isotropic Material

Assigned in: Analysis 1

Property

Young's modulus
Poisson's ratio
Density
Type
Friction angle
Dilation angle
Damping factor
Single cohesion value
Single tensile value
Single compressive value
Type
Model
Calculate initial void ratio
Young's modulus for Ko
Coefficient of lateral earth pressure

Symbol

E
nu
rho
type_8
frc
dil
dmpFct
CSngle
QmaxSngle
QminSngle
type_6
model
calcVoid
Eko
Ko

Value

35000,0
0,3
1,1
"Rankine"
31,0
31,0
0,0
0,0
0,0
1000000000,0
"elastic"
"Elastic"
false
0,0
1,0

~~**Loadcase ID: 1 Title: Loadcase 1**~~

Assignment to Surfaces:

5T8

Attribute: 8 Title: Moran - HW
Sub Type = Isotropic Material
Assigned in: Analysis 1

Property

Young's modulus
Poisson's ratio
Density
Type
Friction angle
Dilation angle
Damping factor
Single cohesion value
Single tensile value
Single compressive value
Type
Model
Calculate initial void ratio
Young's modulus for Ko
Coefficient of lateral earth pressure

Symbol

E
nu
rho
type_8
fric
dil
dmpFct
CSngle
QmaxSngle
QminSngle
type_6
model
calcVoid
Eko
Ko

Value

45000,0
0,3
1,1
"Rankine"
32,0
32,0
0,0
0,0
0,0
1000000000,0
"elastic"
"Elastic"
false
0,0
1,0

~~**Loadcase ID: 1 Title: Loadcase 1**~~
Assignment to Surfaces:
1T4

9. Supports

Attribute: 1 Title: Fixed in X
Sub Type = Structural Support
Assigned in: Analysis 1

Property	Symbol	Value
Translation in X	U	"R"
Translation in Y	V	"F"
Rotation about Z	THZ	"F"
Pore pressure	pore	"C"

~~Loadcase ID: 1~~ ~~Title: Loadcase 1~~ Information “Assigned in” seems sufficient)

Assignment to Lines:
5;9;14;18;23;27;32;36;41;48

Attribute: 2 Title: Fixed in XY
Sub Type = Structural Support
Assigned in: Analysis 1

Property	Symbol	Value
Translation in X	U	"R"
Translation in Y	V	"R"
Rotation about Z	THZ	"F"
Pore pressure	pore	"C"

~~Loadcase ID: 1~~ ~~Title: Loadcase 1~~

Assignment to Lines:
1T4

10. Loading

Attribute: 1 Title: Surface loading

Sub Type = Global Distributed Load

Property

Attribute type

X Direction

Y Direction

Z Direction

Moment about X axis

Moment about Y axis

Moment about Z axis

Moment about hinge nodes

Pore pressure flux

Keep global

Symbol

type

WX

WY

WZ

MX

MY

MZ

Hinge

pwp

keepGlobal

Value

"Length"

60,0

-250,0

0,0

0,0

0,0

0,0

0,0

0,0

false

Loadcase ID: 1 Title: Loadcase 1 Factor = 1.0

Assignment to Lines:

29;30

Gravity (when chosen) is applied as a body force of $-9,8\text{m/s}^2$ in the Y direction

11. Loadcases

Loadcase	Gravity	Loading	Analysis	Branch
1	On	-	Analysis 1	None
2	On	-	Analysis 1	Branch 1: Soil
3	On	1: Surface loading	Analysis 1	Branch 2: Soil + surface load