

Piled design using Swedish P-y curves

|  |                                      |          |             |
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## Appendix

1. STRAIGHT PILE: Input receipt
2. IMPERFECT PILE: Input receipt

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## **1. GENERAL**

A steel pile is evaluated using a non-linear analysis.

The pile pinned a bottom and is unrestrained at top.

The movement of superstructure due to movements results in forced translation ( $\delta_H$ ).

An eccentric imperfection ( $e_{exc}$ ) when installing bearings relative to piles is considered.

Also imperfection of piles ( $\delta_0$ ) is considered.

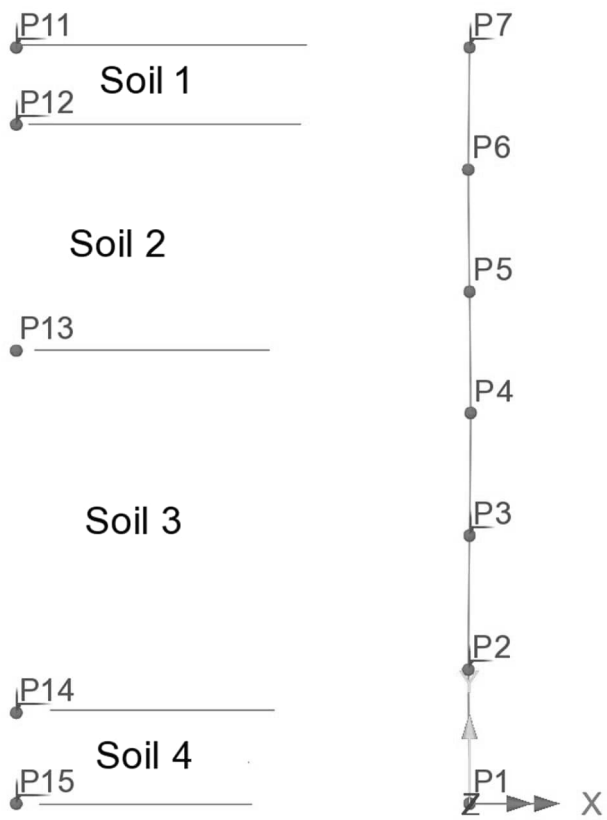
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1.1 OVERVIEW



| Soil | H   | $\gamma$          | $\gamma'$         | Stiffness    | Material        |
|------|-----|-------------------|-------------------|--------------|-----------------|
| 1    | 1.7 | 18                | 10                | "Mycket låg" | "Sand"          |
| 2    | 5.0 | 20                | 13                | "Medelhög"   | "Sandig morän"  |
| 3    | 8.0 | 20                | 13                | "Medelhög"   | "Grusig morän"  |
| 4    | 2.0 | 20                | 13                | "Hög"        | "Blockig morän" |
| -    | m   | kN/m <sup>3</sup> | kN/m <sup>2</sup> | -            | -               |

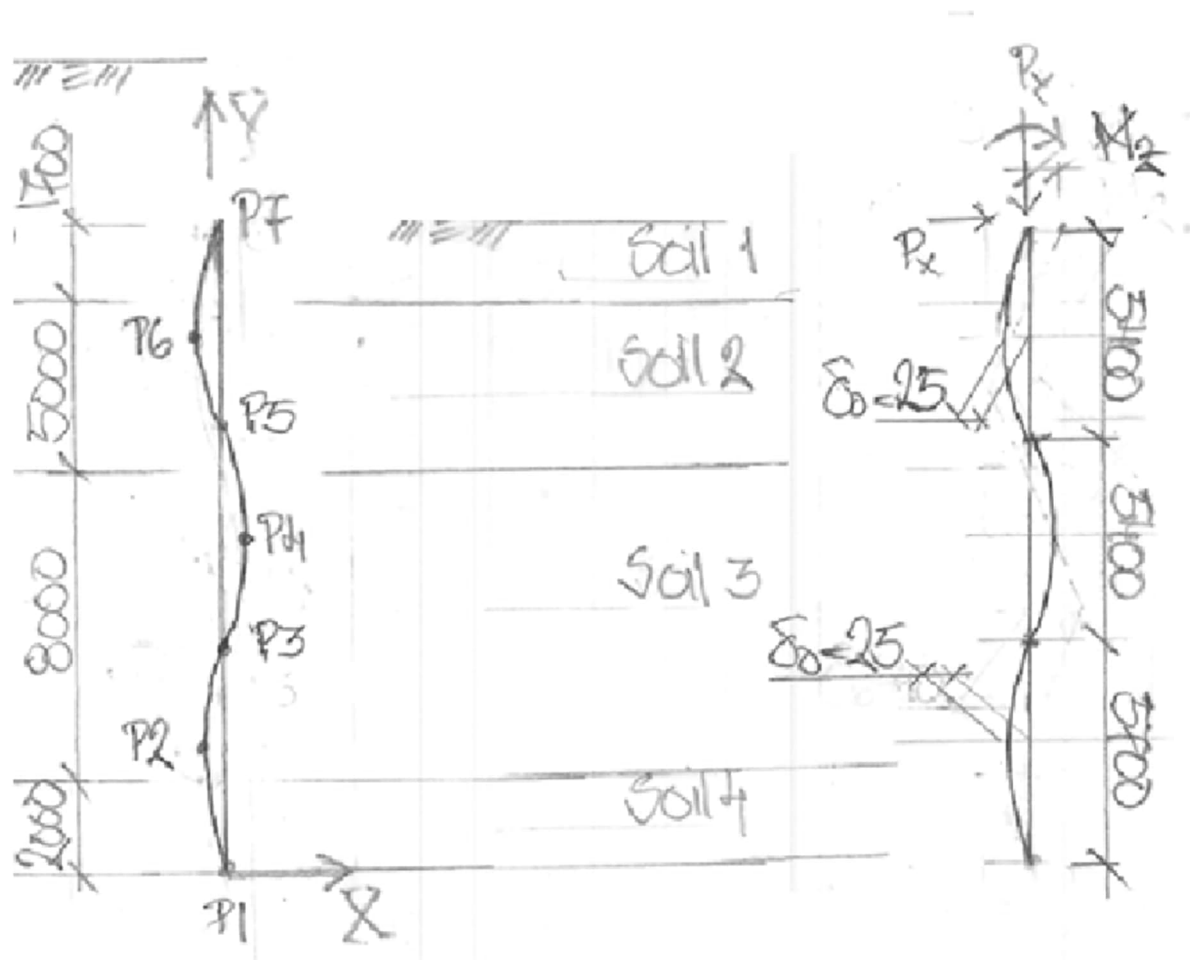
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1.3 PILE IMPERFECTIONS

Assumed pile imperfections.



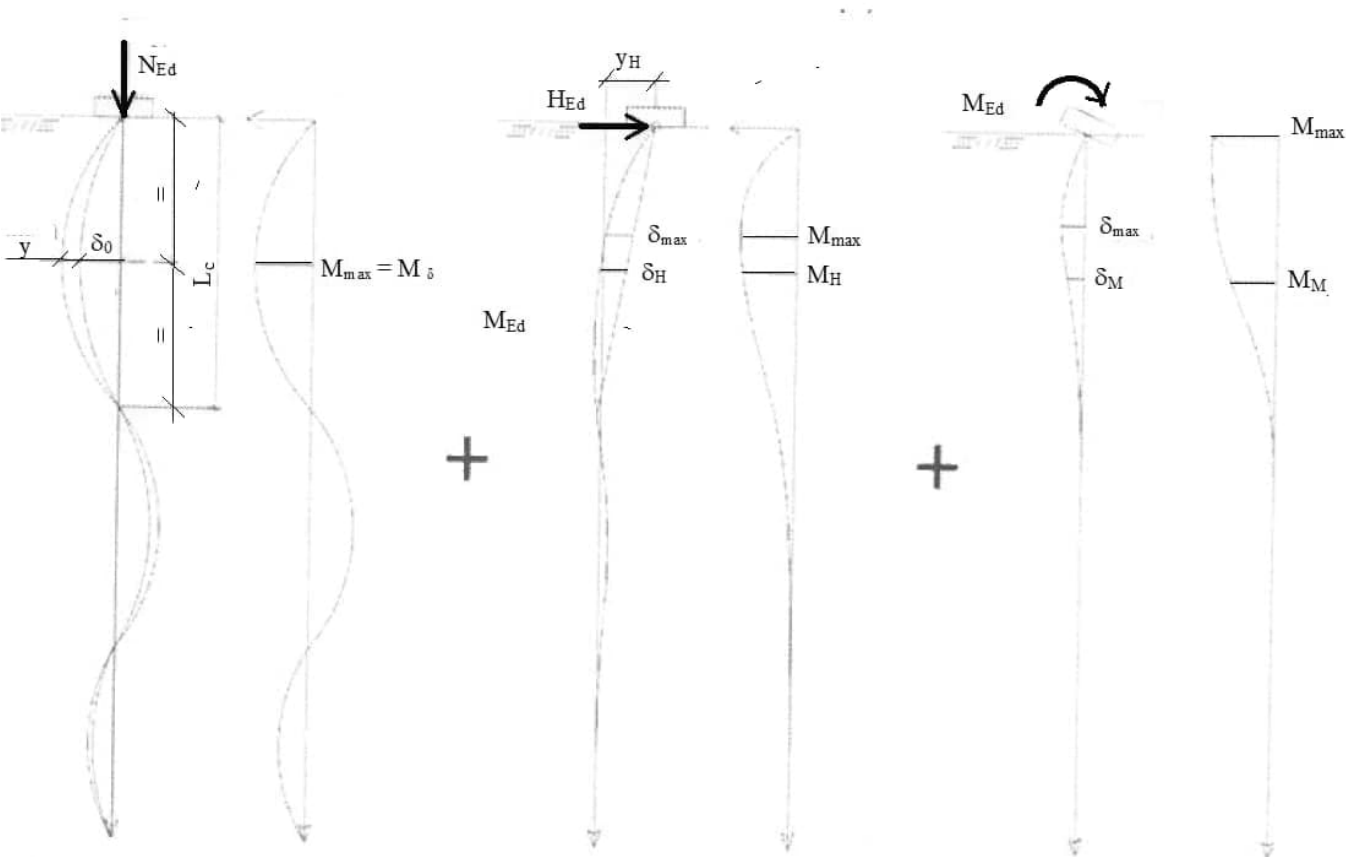
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### 1.4 GEOTECHNICAL THEORY

Below are the effects of initial imperfections ( $\delta_0$ ), horizontal force ( $H_{Ed}$ ) and moment ( $M_{Ed}$ ).  
What is seen is that  $M_{max}$  do not act a same location.

It can also be seen that soil resistance is lowest at top of pile where  $H_{Ed}$  and  $M_{Ed}$  are acting.  
Generally, soil reaches maximal pressure (yield strength) near top of pile since load effect is highest and soil strength is often weakest. Once the load effect reaches yield strength no additional lateral soil force will act, thus initial imperfections will give less contribution to design load effect since  $M_{max}$  is located lower than for horizontal force and moment.

Two analyses will be performed to verify this assumption. The first analysis is without initial imperfections and the second including.



Initial imperfection

Horizontal force  
 $(y_H \rightarrow H_{Ed})$

Moment  
 $(M_{Ed} = N_{Ed} \cdot e_{exc})$

|  |                                      |          |             |
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### 1.5 Swedish P-y curves

P-y curves are determined using TRVINFRA-00227 appendix 3.

Since soil height varies on either side of pile due to embankment both are considered.

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### 1.5.1 Lateral stiffness

$$K = \min(k_k \cdot d; K_{max}) = \min(n_h \cdot z; K_{max}) \left[ \frac{kN}{m^2} \right]$$

#### Excluding embankment

| Soil | z    | n <sub>h</sub>    | K <sub>max</sub>  | n <sub>h</sub> · z | K                 |
|------|------|-------------------|-------------------|--------------------|-------------------|
| 1    | 0    | 1500              | 2.40              | 0                  | 0                 |
| -"-  | 1.7  | 1500              | 2.40              | 2.55               | 2,40              |
| 2    | 1.7  | 4500              | 18.00             | 7.65               | 4,50              |
| -"-  | 4.0  | 4500              | 18.00             | 18.00              | 18,00             |
| -"-  | 4.2  | 4500              | 18.00             | 18.90              | 18.00             |
| -"-  | 6.7  | 4500              | 18.00             | 30.15              | 18,00             |
| 3    | 6.7  | 4500              | 18.00             | 30.15              | 18,00             |
| -"-  | 10.7 | 4500              | 18.00             | 48.15              | 18,00             |
| -"-  | 14.7 | 4500              | 18.00             | 66.15              | 18,00             |
| 4    | 14.7 | 7500              | 30.00             | 66.15              | 30,00             |
| -"-  | 16.7 | 7500              | 30.00             | 125.25             | 30,00             |
| -    | m    | kN/m <sup>3</sup> | kN/m <sup>2</sup> | MN/m <sup>2</sup>  | MN/m <sup>2</sup> |

#### Including embankment (h = 3.8 m)

| Soil | z    | n <sub>h</sub>    | K <sub>max</sub>  | n <sub>h</sub> · z | K                 |
|------|------|-------------------|-------------------|--------------------|-------------------|
| 1    | 3.8  | 1500              | 2.40              | 5.70               | 2,40              |
| -"-  | 5.5  | 1500              | 2.40              | 8.25               | 2,40              |
| 2    | 5.5  | 4500              | 18.00             | 24.75              | 18,00             |
| -"-  | 8.0  | 4500              | 18.00             | 36.00              | 18,00             |
| -"-  | 10.5 | 4500              | 18.00             | 47.25              | 18,00             |
| 3    | 10.5 | 4500              | 18.00             | 47.25              | 18,00             |
| -"-  | 15.5 | 4500              | 18.00             | 69.75              | 18,00             |
| -"-  | 18.5 | 4500              | 18.00             | 83.25              | 18,00             |
| 4    | 18.5 | 7500              | 30.00             | 138.75             | 30,00             |
| -"-  | 20.5 | 7500              | 30.00             | 153.75             | 30,00             |
| -    | m    | kN/m <sup>3</sup> | kN/m <sup>2</sup> | MN/m <sup>2</sup>  | kN/m <sup>2</sup> |

|  |                                      |          |             |
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### 1.5.2 Maximum soil pressure

$$q_k = 3K_p \cdot \sigma'_v \left[ \frac{kN}{m^2} \right]$$

#### Excluding embankment

| Soil | z    | $\phi_k$ | $K_p$             | $\sigma'_v$       | $q_k$             | $F = d \cdot q_k$ |
|------|------|----------|-------------------|-------------------|-------------------|-------------------|
| 1    | 0    | 26.7     | 2.63              | 0                 | 0                 | 0                 |
| "-   | 1.7  | 26.7     | 2.63              | 17                | 134               | 43                |
| 2    | 1.7  | 29.1     | 2.89              | 17                | 147               | 47                |
| "-   | 4.2  | 29.1     | 2.89              | 50                | 429               | 136               |
| "-   | 6.7  | 29.1     | 2.89              | 82                | 710               | 226               |
| 3    | 6.7  | 28.5     | 2.82              | 82                | 693               | 221               |
| "-   | 10.7 | 28.5     | 2.82              | 134               | 1133              | 362               |
| "-   | 14.7 | 28.5     | 2.82              | 186               | 1573              | 502               |
| 4    | 14.7 | 42.0     | 5.05              | 186               | 2817              | 899               |
| "-   | 16.7 | 42.0     | 5.05              | 212               | 3212              | 1025              |
| -    | m    | deg      | kN/m <sup>2</sup> | kN/m <sup>2</sup> | kN/m <sup>2</sup> | kN/m              |

#### Including embankment (h = 3.8 m)

| Soil | z    | $\phi_k$ | $K_p$             | $\sigma'_v$       | $q_k$             | $F = d \cdot q_k$ |
|------|------|----------|-------------------|-------------------|-------------------|-------------------|
| 1    | 3.8  | 26.7     | 2.63              | 76*               | 599               | 191               |
| "-   | 5.5  | 26.7     | 2.63              | 93                | 733               | 234               |
| 2    | 5.5  | 29.1     | 2.89              | 93                | 806               | 257               |
| "-   | 8.0  | 29.1     | 2.89              | 75                | 1088              | 347               |
| "-   | 10.5 | 29.1     | 2.89              | 158               | 1369              | 437               |
| 3    | 10.5 | 28.5     | 2.82              | 158               | 1337              | 427               |
| "-   | 14.5 | 28.5     | 2.82              | 210               | 1777              | 567               |
| "-   | 18.5 | 28.5     | 2.82              | 262               | 2216              | 707               |
| 4    | 18.5 | 42.0     | 5.05              | 262               | 3969              | 1266              |
| "-   | 20.5 | 42.0     | 5.05              | 288               | 4363              | 1392              |
| -    | m    | deg      | kN/m <sup>2</sup> | kN/m <sup>2</sup> | kN/m <sup>2</sup> | kN/m              |

$$* = 3.8 \text{ m} \cdot 20 \frac{kN}{m^3} = 76 kPa$$

|  |                                      |          |              |
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## 2. SYSTEM ANALYSIS

### 2.1 GENERAL

The pile structure is modelled in 2D using FEM-program.

The pile is modelled using 2D beam elements (BMI2).

Soil is modelled using non-linear springs (JNT3).

The pile is considered pinned at bottom and free at top.

Two models are studied as seen below.

| Modell        | Imperfections |
|---------------|---------------|
| Pile_straight | No            |
| Pile_spline   | Yes           |

|  |                                      |          |              |
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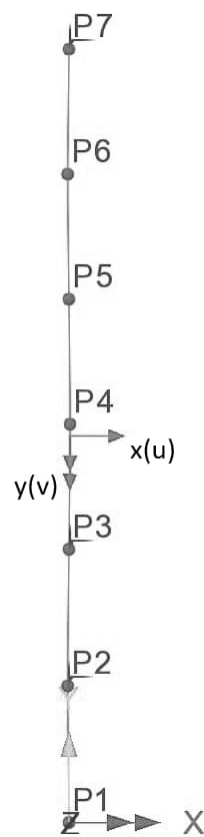
## 2.2 SKETCH SYSTEM ANALYSIS

### 2.2.1 Geometry

Points (P1-P7) and line (L1-L6) including coordinate systems.

*Global: X-Y-Z*

*Local line coordinate system: x-y*



|  |                                      |          |              |
|--|--------------------------------------|----------|--------------|
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2.2.2 Local coordinate system

Due to imperfections the *Local line coordinate system*: x-y can not be used for definition joint elements, thus coordinate system Joint coordinate system is createad.

Local Coordinate

Coordinates type

☒ Cartesian

☐ Cylindrical

☐ Spherical

☐ Surface

☒ Rotate

☐ Scale

☐ Matrix

Angle

90,0

About axis

☐ X-axis

☐ Y-axis

☒ Z-axis

Origin

X

0,0

Y

0,0

Z

0,0

Local coordinate generated from selection

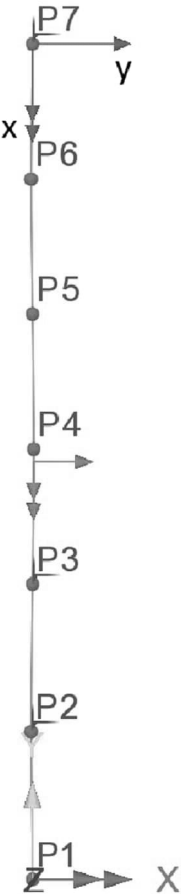
No Local coordinate attributes created from selection

Use

Name

Joint coordinate system

(1)



|  |                                      |          |              |
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### 2.2.3 Cross section properties

The properties include the effect of concrete (C35/45) and corrosion ( $t_r$ ) of pile ( $\phi 323.9 \times 12.5$ ).

$$d = D - 2t_{rm} = 323.9 - 2 \times 2.4 \text{ mm} = 319 \text{ mm}$$

$$A = 9.805 \cdot 10^{-3} \text{ m}^2$$

$$I = 1.406 \cdot 10^{-4} \text{ m}^4$$

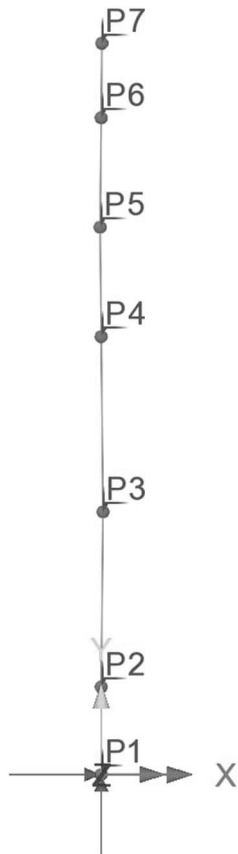
$$E_{sk} = 210 \text{ GPa} \quad \rightarrow E = \frac{\alpha_s \cdot \mu_s \cdot E_{sk}}{\gamma_{M0}} = \frac{0.9 \cdot 1.0 \cdot 210 \text{ GPa}}{1.0} = 189 \text{ GPa}$$

-

|  |                                      |          |              |
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#### 2.2.4 Boundary

Pile is pinned at bottom (P1).



|  |                                      |          |              |
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2.2.5 MESH: Beam element (BMI2)

Pile is modelled as thick beam as seen below.

| Name | Element length | End release: Start | End release: End |
|------|----------------|--------------------|------------------|
| Pile | 0.5 m          | None               | None             |



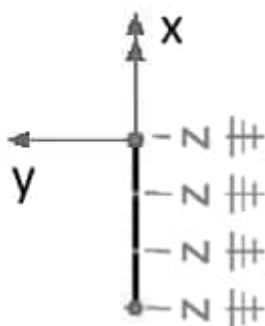
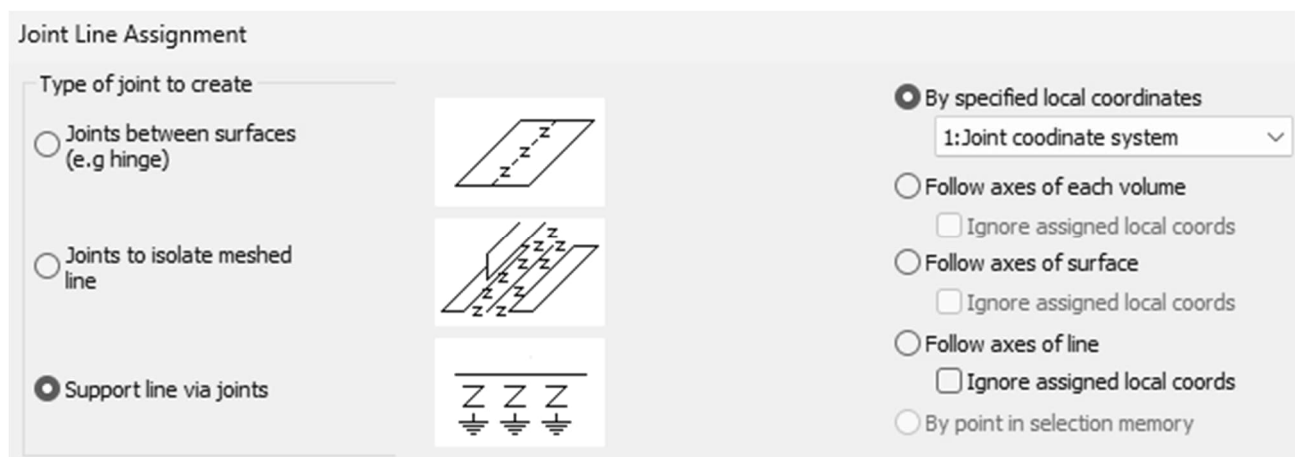
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### 2.2.6 Joint element between lines (JNT3 )

Connection between soil and pile is of type joint element, see presentation below. The joint has no rotational stiffness in order achieve a hinge.

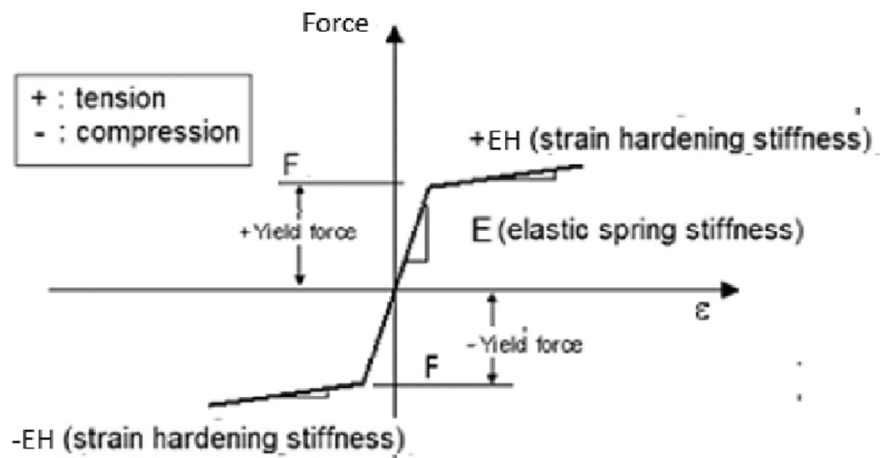
The joint uses nonlinear material termed *Elasto-Plastic (Tension and Compression Equal)*. They vary along pile. This is handled using *Line Variation*.

The joint are introduced as same location as nodes along beam element.



### Principle sketch

|  |                                      |          |              |
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The properties “E” and “F” are vary over distance. This is handled by introducing a line variation.

#### Remark

Since lateral resistance is higher excluding embankment, this is used in system analysis since weaker soil material generates more load effect in pile.

2.2.6.1 Elastic spring stiffness (Elastic spring stiffness  $\therefore$  E)

See page 7 (excluding embankment).

Line Variation

Type Interpolation

☒ Specify order

Order Linear

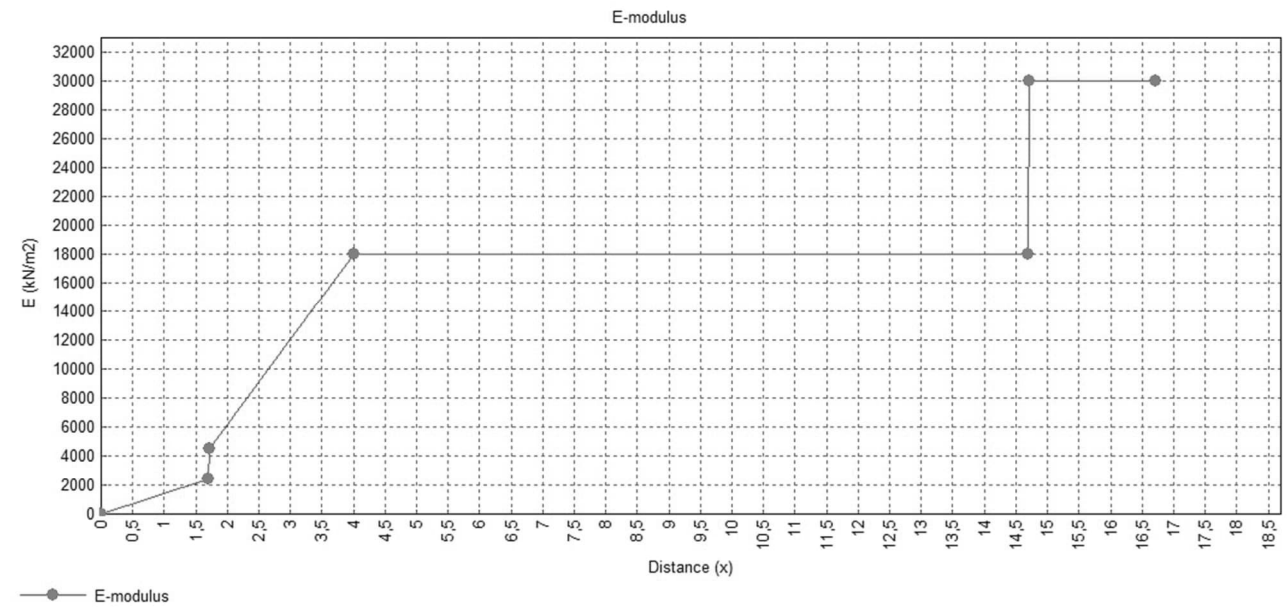
☐ Equal spacing assumed

Distance type Actual

|    | Distance (x) | Value (y) |
|----|--------------|-----------|
| 1  | 0            | 0,0       |
| 2  | 1,7          | 2,4E3     |
| 3  | 1,71         | 4,5E3     |
| 4  | 4            | 18,0E3    |
| 5  | 4,2          | 18,0E3    |
| 6  | 6,7          | 18,0E3    |
| 7  | 6,71         | 18,0E3    |
| 8  | 10,7         | 18,0E3    |
| 9  | 14,7         | 18,0E3    |
| 10 | 14,71        | 30,0E3    |
| 11 | 16,71        | 30,0E3    |

Name E

(1)



|  |                                      |          |              |
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2.2.6.2
Maximum soil pressure (Yield force  $\therefore$  F)

See page 8 (excluding embankment).

Line Variation

Type

Interpolation

☒ Specify order

Order

Linear

☐ Equal spacing assumed

Distance type

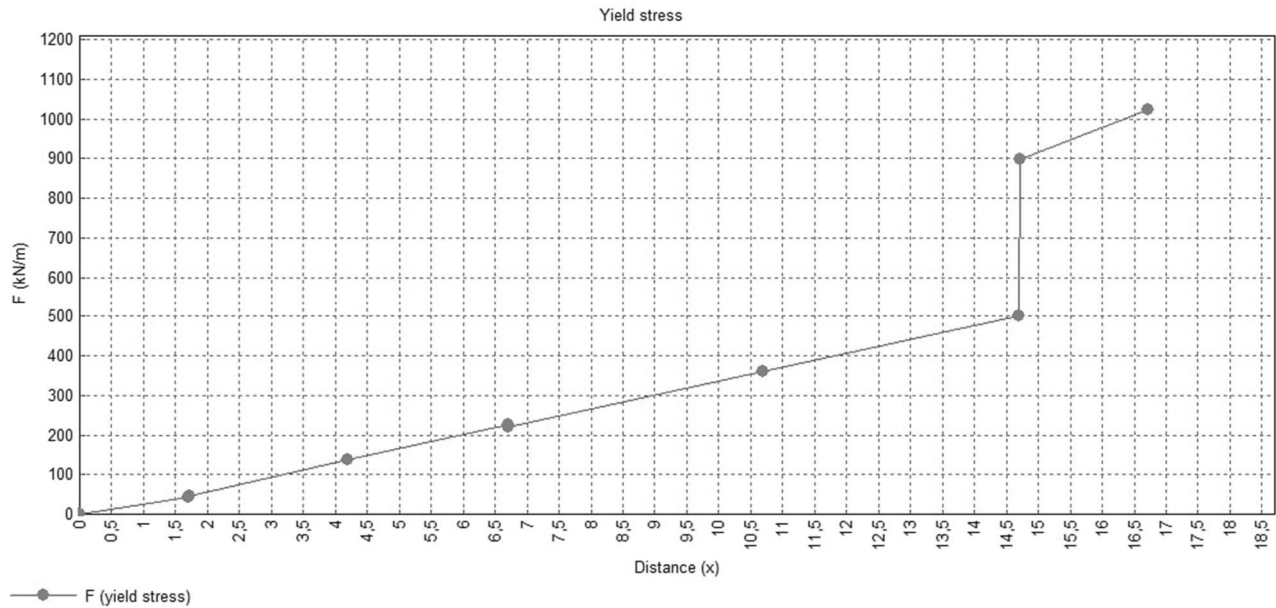
Actual

|    | Distance (x) | Value (y) |
|----|--------------|-----------|
| 1  | 0            | 0,0       |
| 2  | 1,7          | 43,0      |
| 3  | 1,71         | 47,0      |
| 4  | 4,2          | 136,0     |
| 5  | 6,7          | 226,0     |
| 6  | 6,71         | 221,0     |
| 7  | 10,7         | 362,0     |
| 8  | 14,7         | 502,0     |
| 9  | 14,71        | 899,0     |
| 10 | 16,71        | 1,025E3   |

Name

F

(14)

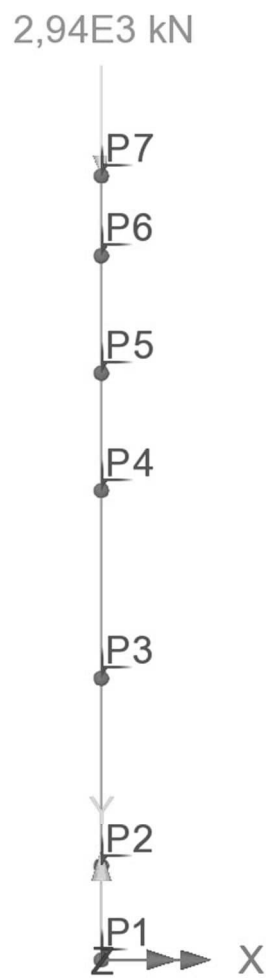


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**3. LOADS**

**3.1 VERTICAL LOAD**

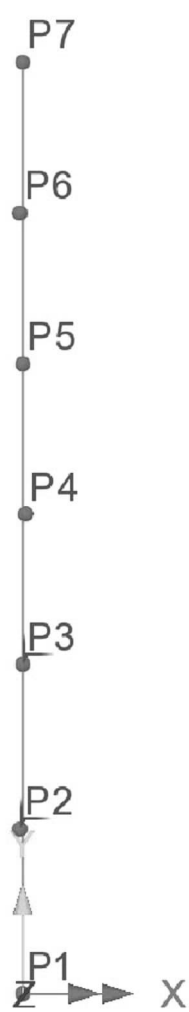
$N_{Ed} = 2940 \text{ kN}$



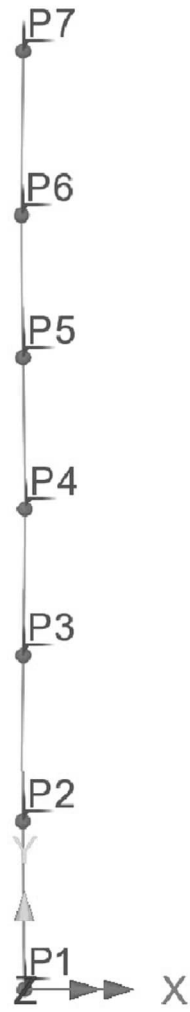
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3.2 INITIAL IMPERFECTIONS

Two models are studied. Either with or without imperfections, see page 3.



Straight pile



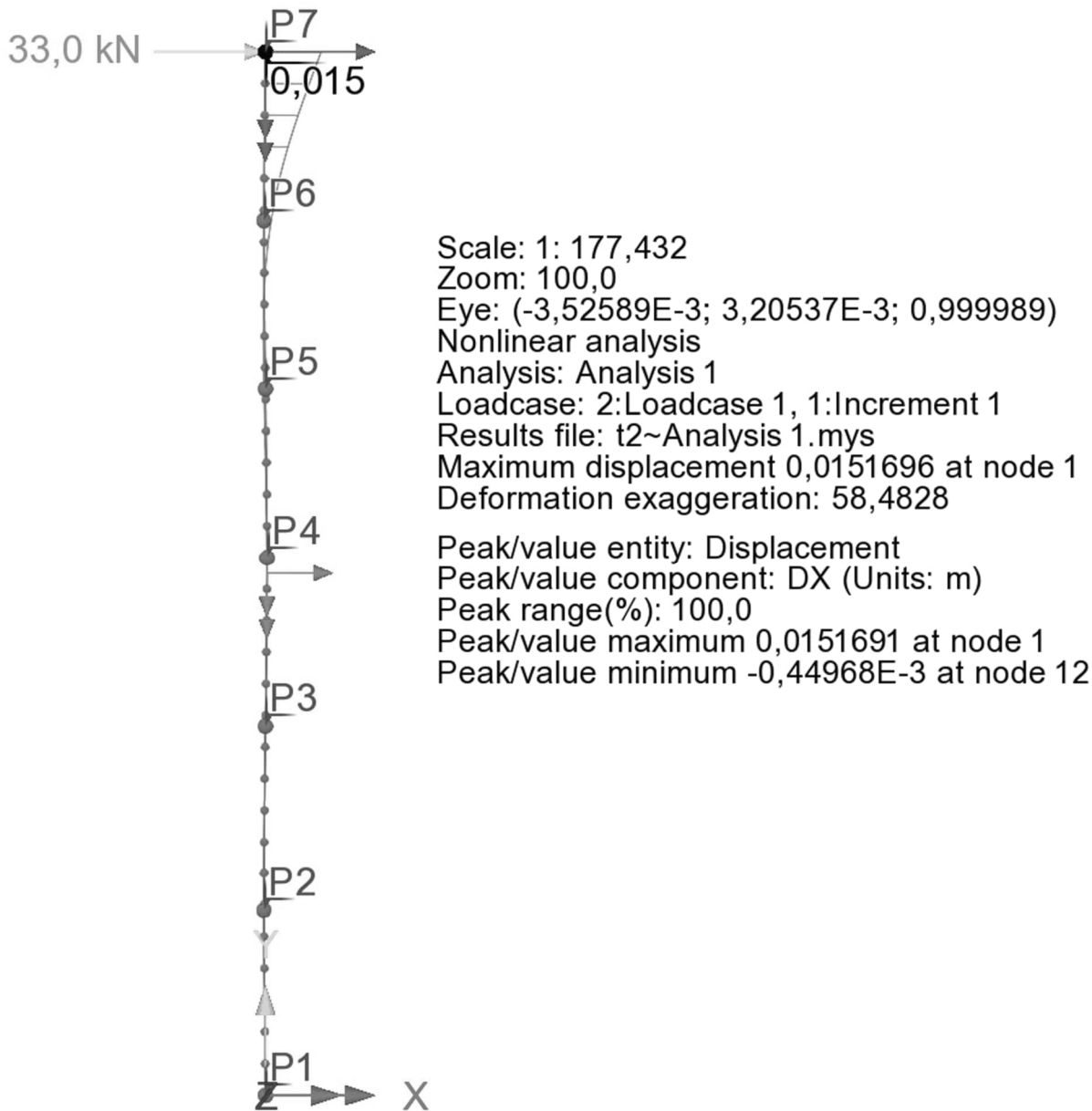
Spline pile

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3.3
HORIZONTAL LOAD

The horizontal load acts due to movement ( $y_H$ ) of superstructure.

$y_H = 15\text{ mm} \rightarrow H_{Ed} = 33\text{ kN}$

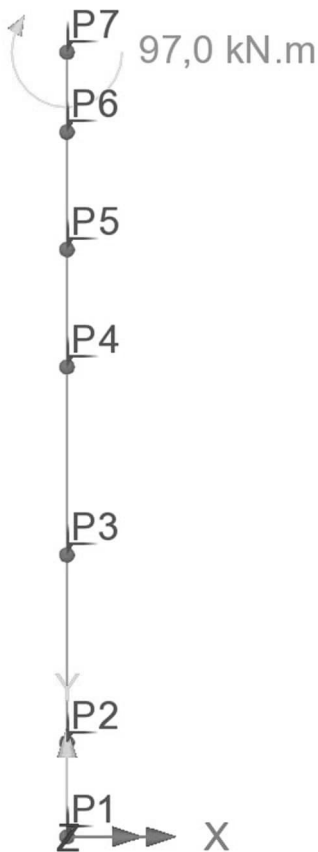


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### 3.4 MOMENT LOAD

The moment load arises due to excentric bearing load.

$$e_{exc} = 33 \text{ mm} \rightarrow M_{Ed} = e_{exc} \cdot N_{Ed} = 0.033 \text{ m} \cdot 2940 \text{ kN} = 97 \text{ kNm}$$



|  |                                      |          |              |
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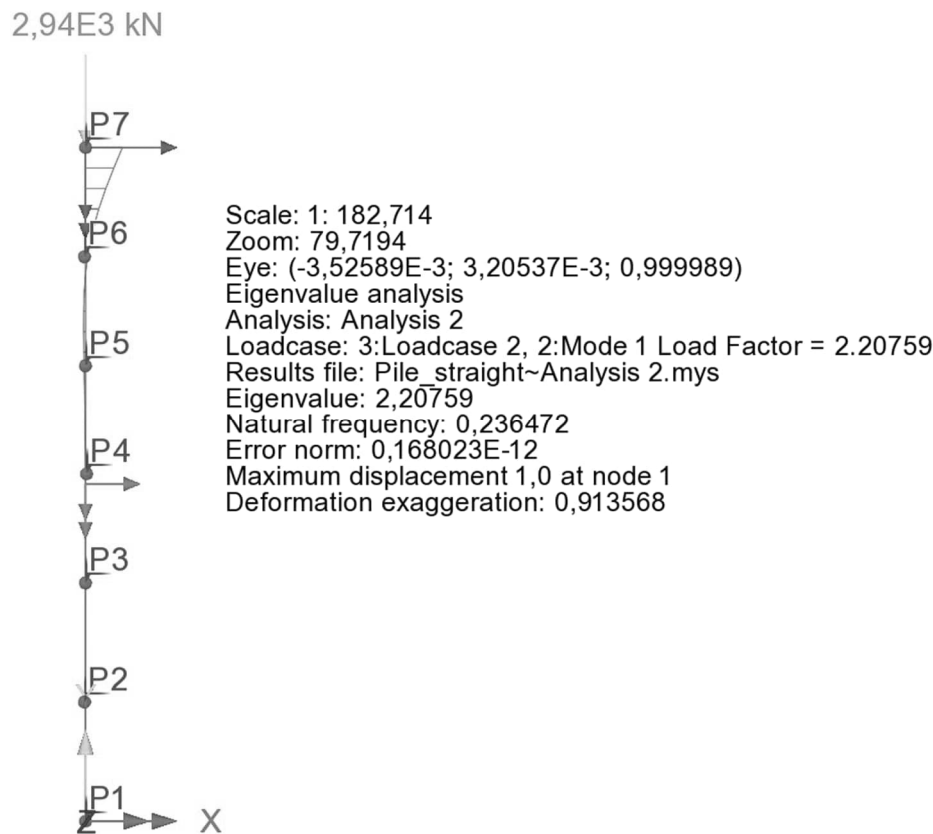
#### 4. SUMMARY RESULTS

##### 4.1 STRAIGHT PILE

See appendix 1 (∴ Pile\_straight).

##### 4.1.1 Buckling load

$$N_{cr} = s \cdot N_{Ed} = 2.208 \cdot 2940 \text{ kN} = 6491 \text{ kN}$$

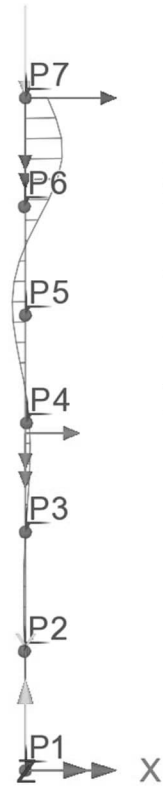


##### Mode 1

Load factor 2.208

|  |                                      |          |              |
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2,94E3 kN

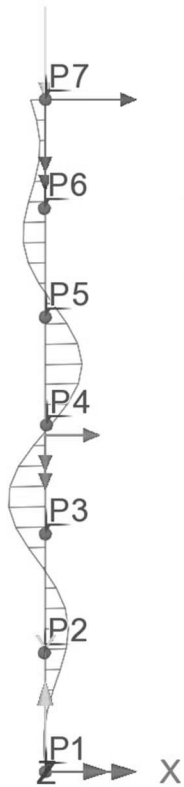


Scale: 1: 182,714  
Zoom: 79,7194  
Eye: (-3,52589E-3; 3,20537E-3; 0,999989)  
Eigenvalue analysis  
Analysis: Analysis 2  
Loadcase: 3:Loadcase 2, 2:Mode 2 Load Factor = 12.1031  
Results file: Pile\_straight~Analysis 2.mys  
Eigenvalue: 12,1031  
Natural frequency: 0,553693  
Error norm: 25,0926E-12  
Maximum displacement 1,0 at node 5  
Deformation exaggeration: 0,913568

Mode 2  
Load factor 12.103

|  |                                      |          |              |
|--|--------------------------------------|----------|--------------|
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2,94E3 kN



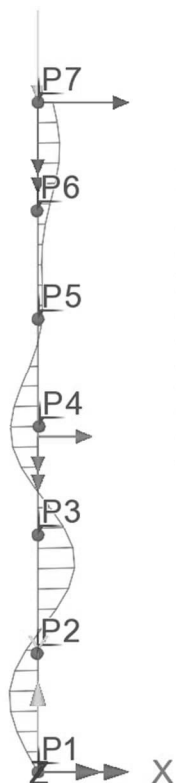
Scale: 1: 182,714  
Zoom: 79,7194  
Eye: (-3,52589E-3; 3,20537E-3; 0,999989)  
Eigenvalue analysis  
Analysis: Analysis 2  
Loadcase: 3:Loadcase 2, 2:Mode 3 Load Factor = 15.8698  
Results file: Pile\_straight~Analysis 2.mys  
Eigenvalue: 15,8698  
Natural frequency: 0,634024  
Error norm: 2,55508E-9  
Maximum displacement 1,0 at node 15  
Deformation exaggeration: 0,913568

### Mode 3

Load factor 15.870

|  |                                      |          |              |
|--|--------------------------------------|----------|--------------|
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|  |                                      | Date :   | Created :    |

2,94E3 kN



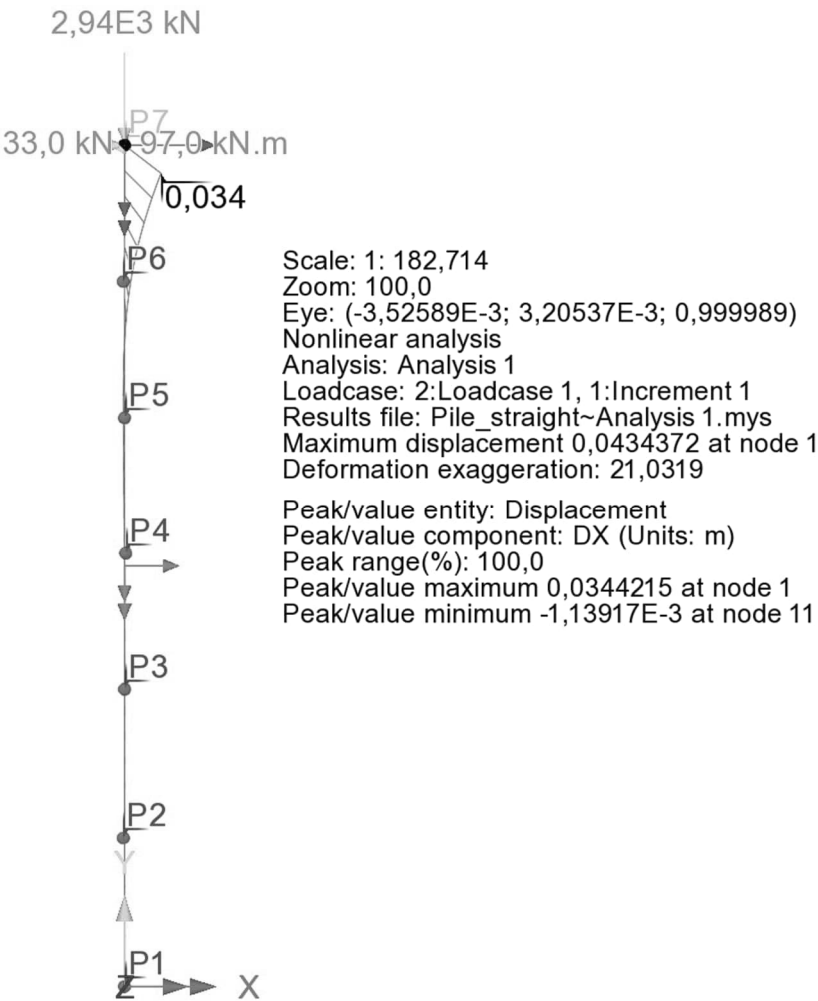
Scale: 1: 182,714  
Zoom: 79,7194  
Eye: (-3,52589E-3; 3,20537E-3; 0,999989)  
Eigenvalue analysis  
Analysis: Analysis 2  
Loadcase: 3:Loadcase 2, 2:Mode 4 Load Factor = 16.1768  
Results file: Pile\_straight~Analysis 2.mys  
Eigenvalue: 16,1768  
Natural frequency: 0,640128  
Error norm: 25,5365E-9  
Maximum displacement 1,0 at node 25  
Deformation exaggeration: 0,913568

Mode 4  
Load factor 16.177

|  |                                      |          |              |
|--|--------------------------------------|----------|--------------|
|  | Pile design using Swedish P-y curves | Status : | Page :<br>28 |
|  |                                      | Date :   | Created :    |

4.1.2            Deformations

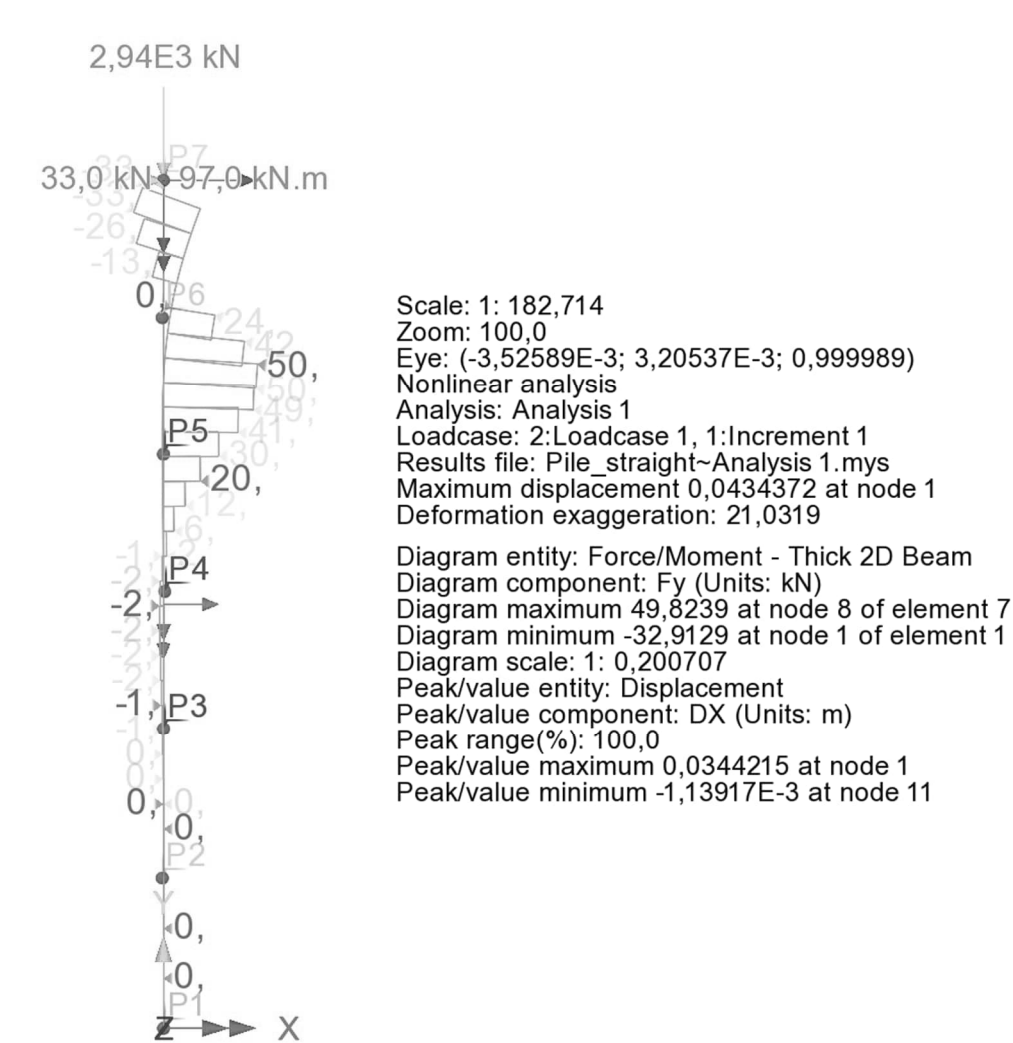
$N_{Ed} = 2940kN, H_{Ed} = 33kN \text{ \& } M_{Ed} = 97kNm \rightarrow max Dx = 34 \text{ mm}$



|  |                                      |          |              |
|--|--------------------------------------|----------|--------------|
|  | Pile design using Swedish P-y curves | Status : | Page :<br>29 |
|  |                                      | Date :   | Created :    |

4.1.3
Shear forces:  $F_y$

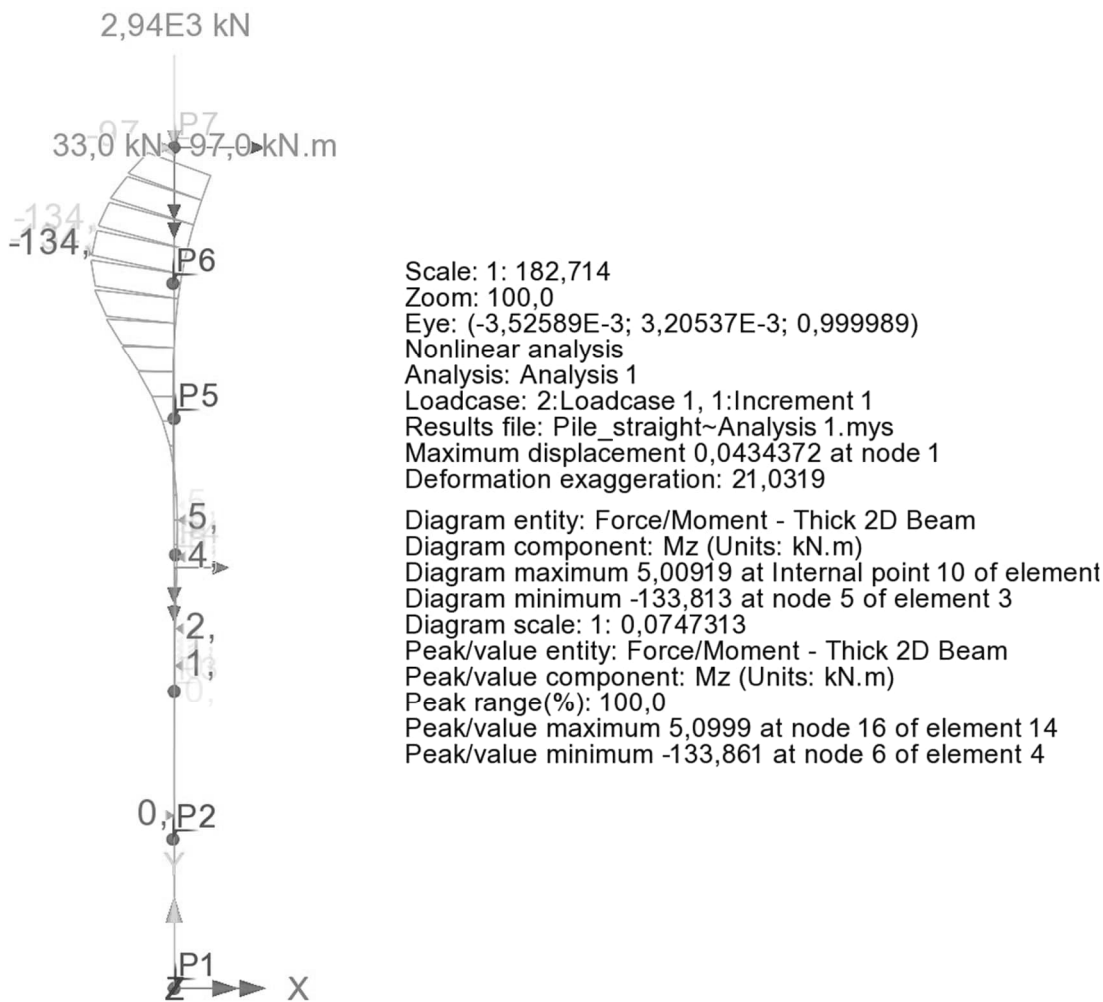
$$N_{Ed} = 2940\text{kN}, H_{Ed} = 33\text{kN} \ \& \ M_{Ed} = 97\text{kNm} \ \rightarrow \max F_y = 50 \text{ kN}$$



|  |                                      |          |              |
|--|--------------------------------------|----------|--------------|
|  | Pile design using Swedish P-y curves | Status : | Page :<br>30 |
|  |                                      | Date :   | Created :    |

4.1.4
Moment:  $M_z$

$N_{Ed} = 2940kN, H_{Ed} = 33kN \text{ \& } M_{Ed} = 97kNm \rightarrow max M_z = 134 kNm$



|  |                                      |          |              |
|--|--------------------------------------|----------|--------------|
|  | Pile design using Swedish P-y curves | Status : | Page :<br>31 |
|  |                                      | Date :   | Created :    |

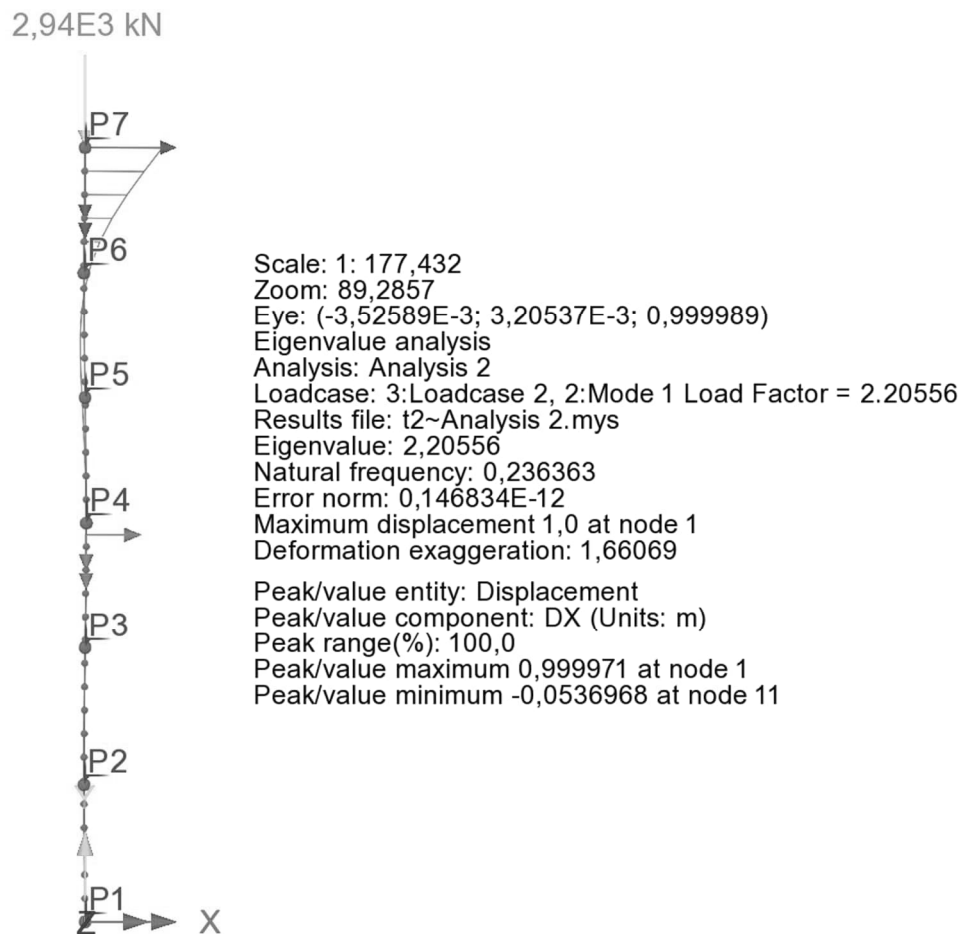
## 4.2 INITIALLY IMPERFECT PILE

See appendix 2 (∴ Pile\_spline).

### 4.2.1 Buckling load

The buckling load is almost the same as straight pile, see value inside parenthesis.

$$N_{cr} = s \cdot N_{Ed} = 2.206 \cdot 2940 \text{ kN} = 6485 \text{ kN} \text{ (6491 kN)}$$



### Mode 1

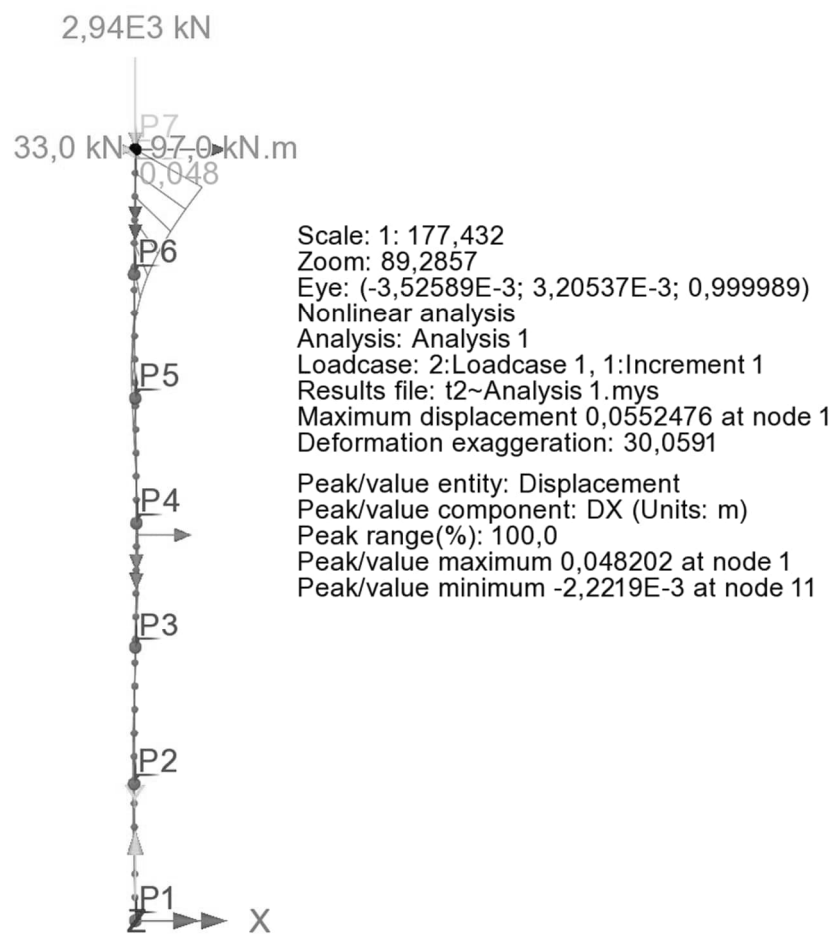
Load factor 2.206

|  |                                      |          |              |
|--|--------------------------------------|----------|--------------|
|  | Pile design using Swedish P-y curves | Status : | Page :<br>32 |
|  |                                      | Date :   | Created :    |

4.2.2
Deformations

The deformation is almost twice as straight pile, see value inside parenthesis.

$N_{Ed} = 2940kN, H_{Ed} = 33kN \text{ \& } M_{Ed} = 97kNm \rightarrow max Dx = 48 \text{ mm } (34 \text{ mm})$

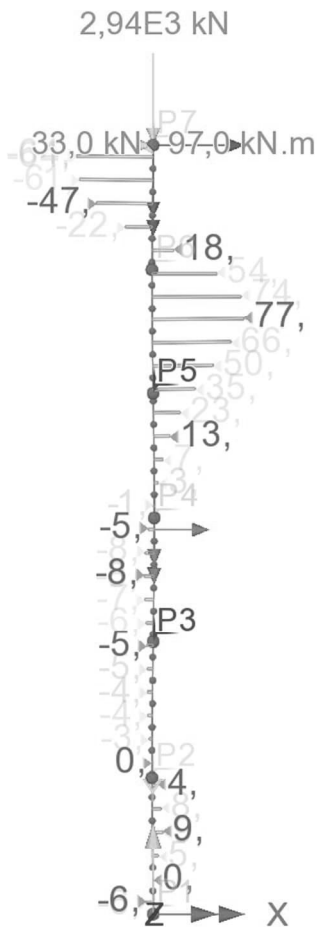


|  |                                      |          |              |
|--|--------------------------------------|----------|--------------|
|  | Pile design using Swedish P-y curves | Status : | Page :<br>33 |
|  |                                      | Date :   | Created :    |

4.2.3 Shear forces:  $F_y$

The shear forces more than twice as straight pile, see value inside parenthesis.

$N_{Ed} = 2940\text{kN}, H_{Ed} = 33\text{kN} \ \& \ M_{Ed} = 97\text{kNm} \ \rightarrow \max F_y = 77\text{ kN} \ (50\text{ kN})$

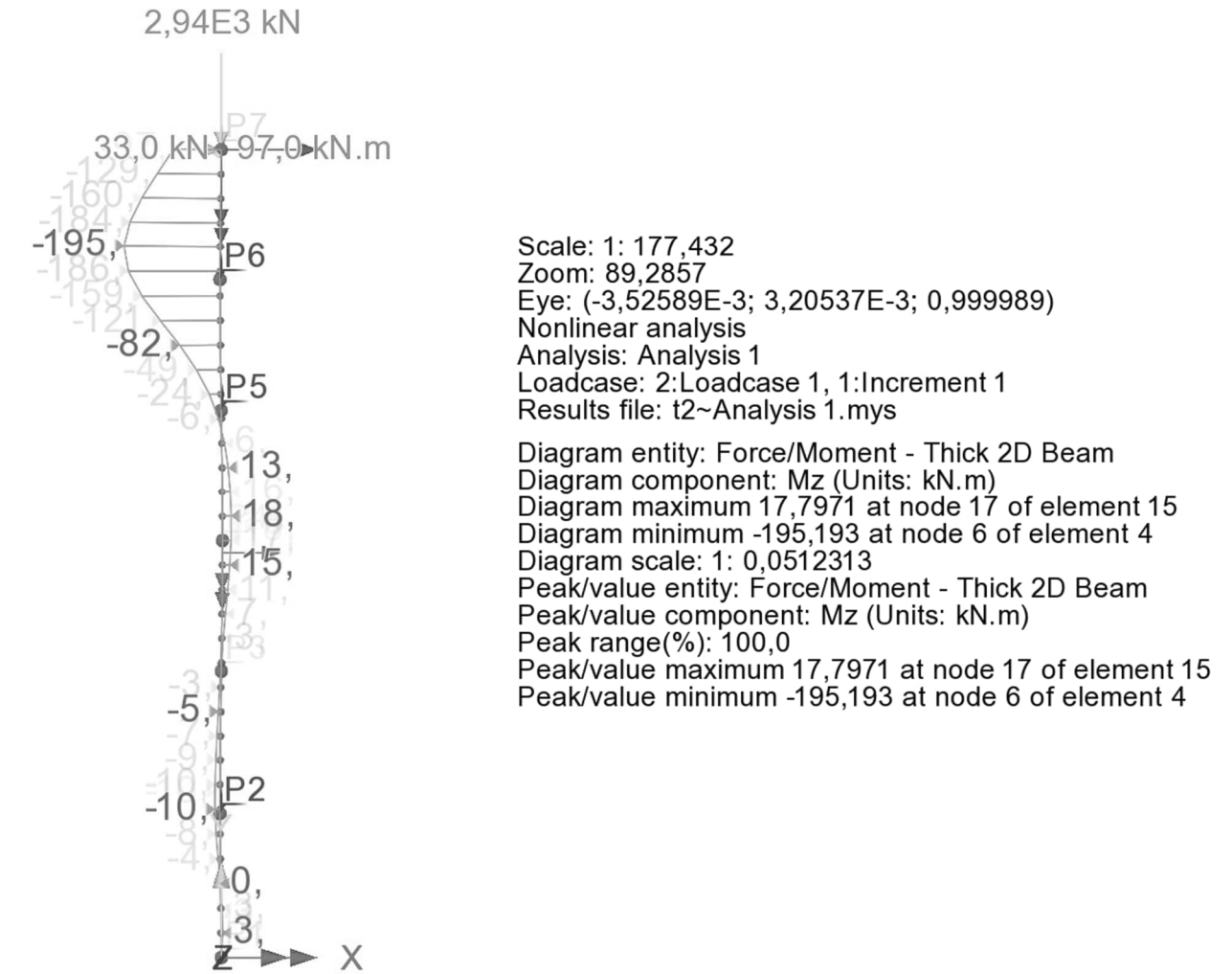


Scale: 1: 177,432  
Zoom: 89,2857  
Eye: (-3,52589E-3; 3,20537E-3; 0,999989)  
Nonlinear analysis  
Analysis: Analysis 1  
Loadcase: 2:Loadcase 1, 1:Increment 1  
Results file: t2~Analysis 1.mys  
Diagram entity: Force/Moment - Thick 2D Beam  
Diagram component:  $F_y$  (Units: kN)  
Diagram maximum 76,5913 at node 9 of element 8  
Diagram minimum -63,6881 at node 1 of element 1  
Diagram scale: 1: 0,130563  
Peak/value entity: Force/Moment - Thick 2D Beam  
Peak/value component:  $F_y$  (Units: kN)  
Peak range(%): 100,0  
Peak/value maximum 75,495 at node 9 of element 7  
Peak/value minimum -63,6881 at node 1 of element 1

|  |                                      |          |              |
|--|--------------------------------------|----------|--------------|
|  | Pile design using Swedish P-y curves | Status : | Page :<br>34 |
|  |                                      | Date :   | Created :    |

4.2.4                   Moment: Mz

$N_{Ed} = 2940kN, H_{Ed} = 33kN$  &  $M_{Ed} = 97kNm \rightarrow max Mz = 195kNm$  (134 kNm)



## Title: Input receipt

**LUSAS version:** 23.0-1c3

**Model Units:** kN,m,t,s,C

**Report Units:** kN,m,t,s,C

**Model Title:**

**Model File:** Pile straight

## APPENDIX 1

### Table of Contents

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| Loadcase         | 10 |

## APPENDIX 1

### Points

| Point | X coordinate | Y coordinate | Z coordinate |
|-------|--------------|--------------|--------------|
| 1     | 0,000        | 0,000        | 0,000        |
| 2     | -0,025       | 2,950        | 0,000        |
| 3     | 0,000        | 5,900        | 0,000        |
| 4     | 0,025        | 8,600        | 0,000        |
| 5     | 0,000        | 11,300       | 0,000        |
| 6     | -0,025       | 14,000       | 0,000        |
| 7     | 0,000        | 16,700       | 0,000        |
| 11    | -10,000      | 16,700       | 0,000        |
| 12    | -10,000      | 15,000       | 0,000        |
| 13    | -10,000      | 10,000       | 0,000        |
| 14    | -10,000      | 2,000        | 0,000        |
| 15    | -10,000      | 0,000        | 0,000        |

## APPENDIX 1

### Lines

| Line | Points | Line | Points |
|------|--------|------|--------|
| 1    | 7;1    |      |        |

## Line Mesh (Elements showing results)

**Attribute: 2 Title: Pile**

Sub Type = Line Mesh Element Type = BMI2

| Mesh spacing | Element size | Start node end releases: | End node end releases: |
|--------------|--------------|--------------------------|------------------------|
| Irregular    | 0,5          | None                     | None                   |

Assignment to Lines:

1

**Attribute: 4 Title: Joint element: soil**

Sub Type = Line Mesh Element Type = JNT3

| Mesh spacing | Start node end releases: | End node end releases: |
|--------------|--------------------------|------------------------|
| Uniform      | None                     | None                   |

Assignment to Lines: Local Coordinates = Joint coordinate system (Local x is joint x), Single feature insertion type : grounded

1

# APPENDIX 1

**Attribute: 1 Title: Pile**

**Sub Type = Line Geometric**

**Assigned in: Analysis 1**

| Property                                                        | Symbol        | Value           |
|-----------------------------------------------------------------|---------------|-----------------|
| Cross sectional area                                            | A             | 0,010           |
| Second moment of area about y axis                              | Iyy           | 0,000           |
| Second moment of area about z axis                              | Izz           | 0,000           |
| Product moment of area                                          | Iyz           | 0,000           |
| Torsional constant                                              | J             | 0,000           |
| Eccentricity in local z direction, relative to specified origin | ez0           | 0,000           |
| Eccentricity in local y direction, relative to specified origin | ey0           | 0,000           |
| Eccentricity in local z direction, relative to beam centroid    | ez            | 0,000           |
| Eccentricity in local y direction, relative to beam centroid    | ey            | 0,000           |
| Wagner constant 1st moment of square radius about y (Iyr)       | Iyr           | 0,000           |
| Wagner constant 1st moment of square radius about z (Izr)       | Izr           | 0,000           |
| Wagner constant 4th moment of area about origin (Irr)           | Irr           | 0,000           |
| Wagner constant 2nd moment of warping about origin (Iwr)        | Iwr           | 0,000           |
| Effective shear area in local z direction                       | Asz           | 0,000           |
| Effective shear area in local y direction                       | Asy           | 0,010           |
| Radius of gyration about y axis                                 | ky            | 0,000           |
| Radius of gyration about z axis                                 | kz            | 0,120           |
| y axis extreme fibre, top                                       | yt            | 0,000           |
| y axis extreme fibre, bottom                                    | yb            | 0,000           |
| z axis extreme fibre, top                                       | zt            | 0,000           |
| z axis extreme fibre, bottom                                    | zb            | 0,000           |
| Shape code identifier                                           | Type          | -1              |
| Element type                                                    | elementType   | "2D Thick Beam" |
| Reinforcement                                                   | reinforcement | None            |

**Assignment to Lines:**

1

## APPENDIX 1

### Attribute: 1 Title: Steel

Sub Type = Isotropic Material

Assigned in: Analysis 1

| Property        | Symbol | Value         |
|-----------------|--------|---------------|
| Young's modulus | E      | 189000000,000 |
| Poisson's ratio | nu     | 0,300         |
| Density         | rho    | 0,000         |

Loadcase ID: 2 Title: Loadcase 1

Assignment to Lines:

1

### Attribute: 2 Title: Joint material: Soil

Sub Type = Joint Material, Elasto-Plastic (Tension and Compression Equal)

Assigned in: Analysis 1

|                              | u            | v                                                     |
|------------------------------|--------------|-------------------------------------------------------|
| Number of degrees of freedom | nDOF         | 2                                                     |
| Joint type                   | JointType    | "Plane stress / plane strain / 2D axisymmetric membra |
| Assignment type              | Assignment   | "Line"                                                |
| Yield force                  | F[0]         | 0,000                                                 |
| Yield force                  | F[1]         | F                                                     |
| Hardening stiffness          | H[0]         | 0,000                                                 |
| Hardening stiffness          | H[1]         | 10,000                                                |
| Elastic spring stiffness     | K[0]         | 0,000                                                 |
| Elastic spring stiffness     | K[1]         | E                                                     |
| Mass                         | M[0]         | 0,000                                                 |
| Mass                         | M[1]         | 0,000                                                 |
| Mass position                | MassPosition | "Between nodes"                                       |

Loadcase ID: 2 Title: Loadcase 1

Assignment to Lines:

1

### Attribute: 1 Title: E

Sub Type = Interpolation Variation

| Property            | Symbol     | Value    |
|---------------------|------------|----------|
| Interpolation order | type       | "Linear" |
| Distance type       | distType   | "Actual" |
| Distance            | distance   | 0,000    |
| Distance            | distance   | 1,700    |
| Distance            | distance   | 1,710    |
| Distance            | distance   | 4,000    |
| Distance            | distance   | 4,200    |
| Distance            | distance   | 6,700    |
| Distance            | distance   | 6,710    |
| Distance            | distance   | 10,700   |
| Distance            | distance   | 14,700   |
| Distance            | distance   | 14,710   |
| Distance            | distance   | 16,710   |
| Function            | expression | "0.0"    |
| Function            | expression | "2.4E3"  |
| Function            | expression | "4.5E3"  |
| Function            | expression | "18.0E3" |
| Function            | expression | "18.0E3" |
| Function            | expression | "18.0E3" |
| Function            | expression | "18.0E3" |
| Function            | expression | "18.0E3" |
| Function            | expression | "30.0E3" |
| Function            | expression | "30.0E3" |

**Attribute: 4   Title: F**  
**Sub Type = Interpolation Variation**

| Property            | Symbol     | Value     |
|---------------------|------------|-----------|
| Interpolation order | type       | "Linear"  |
| Distance type       | distType   | "Actual"  |
| Distance            | distance   | 0,000     |
| Distance            | distance   | 1,700     |
| Distance            | distance   | 1,710     |
| Distance            | distance   | 4,200     |
| Distance            | distance   | 6,700     |
| Distance            | distance   | 6,710     |
| Distance            | distance   | 10,700    |
| Distance            | distance   | 14,700    |
| Distance            | distance   | 14,710    |
| Distance            | distance   | 16,710    |
| Function            | expression | "0.0"     |
| Function            | expression | "43.0"    |
| Function            | expression | "47.0"    |
| Function            | expression | "136.0"   |
| Function            | expression | "226.0"   |
| Function            | expression | "221.0"   |
| Function            | expression | "362.0"   |
| Function            | expression | "502.0"   |
| Function            | expression | "899.0"   |
| Function            | expression | "1.025E3" |

## APPENDIX 1

**Attribute: 1    Title: Pinned**

**Sub Type = Structural Support**

**Assigned in: Analysis 1**

| Property         | Symbol | Value |
|------------------|--------|-------|
| Translation in X | U      | "R"   |
| Translation in Y | V      | "R"   |
| Rotation about Z | THZ    | "F"   |
| Pore pressure    | pore   | "C"   |

**Loadcase ID: 2    Title: Loadcase 1**

**Assignment to Points:**

1

## APPENDIX 1

### Attribute: 3 Title: Lager krafter

Sub Type = Concentrated Load

| Property                   | Symbol       | Value    |
|----------------------------|--------------|----------|
| Concentrated load in X Dir | PX           | 2940,000 |
| Concentrated load in Y Dir | PY           | 33,000   |
| Concentrated load in Z Dir | PZ           | 0,000    |
| Moment about X axis        | MX           | 0,000    |
| Moment about Y axis        | MY           | 0,000    |
| Moment about Z axis        | MZ           | -97,000  |
| Moment about hinge nodes   | L2           | 0,000    |
| Pore pressure flux         | PorePressure | 0,000    |
| Bi moment                  | Mb           | 0,000    |

Loadcase ID: 2 Title: Loadcase 1 Factor = 1.0

Assignment to Points:

7

### Attribute: 9 Title: N = 2940 kN

Sub Type = Concentrated Load

| Property                   | Symbol       | Value    |
|----------------------------|--------------|----------|
| Concentrated load in X Dir | PX           | 2940,000 |
| Concentrated load in Y Dir | PY           | 0,000    |
| Concentrated load in Z Dir | PZ           | 0,000    |
| Moment about X axis        | MX           | 0,000    |
| Moment about Y axis        | MY           | 0,000    |
| Moment about Z axis        | MZ           | 0,000    |
| Moment about hinge nodes   | L2           | 0,000    |
| Pore pressure flux         | PorePressure | 0,000    |
| Bi moment                  | Mb           | 0,000    |

Loadcase ID: 3 Title: Loadcase 2 Factor = 1.0

Assignment to Points:

7

## APPENDIX 1

### Attribute: 1 Title: Joint coordinate system

Sub Type = Local Coordinates

| Property       | Symbol       | Value       |
|----------------|--------------|-------------|
| Attribute type | type         | "Cartesian" |
| Rotation angle | angle        | 270,000     |
| Origin         | originPos[0] | 0,000       |
| Origin         | originPos[1] | 0,000       |
| Origin         | originPos[2] | 0,000       |
| Type           | plane        | "XY_Angle"  |

Assignment to Points:

7

## APPENDIX 1

### Loadcases

| Loadcase | Gravity | Title      | Analysis   | Type |
|----------|---------|------------|------------|------|
| 2        |         | Loadcase 1 | Analysis 1 |      |
| 3        |         | Loadcase 2 | Analysis 2 |      |

Gravity (when chosen) is applied as a body force of  $-9,81000\text{m/s}^2$  in the Y direction

## Title: Input receipt

**LUSAS version:** 23.0-1c3  
**Model Units:** kN,m,t,s,C  
**Report Units:** kN,m,t,s,C

**Model Title:**  
**Model File:** Pile spline

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**Points**

| Point | X coordinate | Y coordinate | Z coordinate |
|-------|--------------|--------------|--------------|
| 1     | 0,000        | 0,000        | 0,000        |
| 2     | -0,025       | 2,950        | 0,000        |
| 3     | 0,000        | 5,900        | 0,000        |
| 4     | 0,025        | 8,600        | 0,000        |
| 5     | 0,000        | 11,300       | 0,000        |
| 6     | -0,025       | 14,000       | 0,000        |
| 7     | 0,000        | 16,700       | 0,000        |
| 11    | -10,000      | 16,700       | 0,000        |
| 12    | -10,000      | 15,000       | 0,000        |
| 13    | -10,000      | 10,000       | 0,000        |
| 14    | -10,000      | 2,000        | 0,000        |
| 15    | -10,000      | 0,000        | 0,000        |

## Splines

| Splines | Points        | Splines | Points |
|---------|---------------|---------|--------|
| 1       | 1;2;3;4;5;6;7 | -       | -      |

## Line Mesh (Elements showing results)

**Attribute: 2 Title: Pile**

Sub Type = Line Mesh Element Type = BMI2

| Mesh spacing | Element size | Start node end releases: | End node end releases: |
|--------------|--------------|--------------------------|------------------------|
| Irregular    | 0,5          | None                     | None                   |

Assignment to Lines:

1

**Attribute: 4 Title: Joint element: soil**

Sub Type = Line Mesh Element Type = JNT3

| Mesh spacing | Start node end releases: | End node end releases: |
|--------------|--------------------------|------------------------|
| Uniform      | None                     | None                   |

Assignment to Lines: Local Coordinates = Joint coordinate system (Local x is joint x), Single feature insertion type : grounded

1

## APPENDIX 2

**Attribute: 1    Title: Pile**

**Sub Type = Line Geometric**

**Assigned in: Analysis 1**

| Property                                                        | Symbol        | Value           |
|-----------------------------------------------------------------|---------------|-----------------|
| Cross sectional area                                            | A             | 0,010           |
| Second moment of area about y axis                              | Iyy           | 0,000           |
| Second moment of area about z axis                              | Izz           | 0,000           |
| Product moment of area                                          | Iyz           | 0,000           |
| Torsional constant                                              | J             | 0,000           |
| Eccentricity in local z direction, relative to specified origin | ez0           | 0,000           |
| Eccentricity in local y direction, relative to specified origin | ey0           | 0,000           |
| Eccentricity in local z direction, relative to beam centroid    | ez            | 0,000           |
| Eccentricity in local y direction, relative to beam centroid    | ey            | 0,000           |
| Wagner constant 1st moment of square radius about y (Iyr)       | Iyr           | 0,000           |
| Wagner constant 1st moment of square radius about z (Izr)       | Izr           | 0,000           |
| Wagner constant 4th moment of area about origin (Irr)           | Irr           | 0,000           |
| Wagner constant 2nd moment of warping about origin (Iwr)        | Iwr           | 0,000           |
| Effective shear area in local z direction                       | Asz           | 0,000           |
| Effective shear area in local y direction                       | Asy           | 0,010           |
| Radius of gyration about y axis                                 | ky            | 0,000           |
| Radius of gyration about z axis                                 | kz            | 0,120           |
| y axis extreme fibre, top                                       | yt            | 0,000           |
| y axis extreme fibre, bottom                                    | yb            | 0,000           |
| z axis extreme fibre, top                                       | zt            | 0,000           |
| z axis extreme fibre, bottom                                    | zb            | 0,000           |
| Shape code identifier                                           | Type          | -1              |
| Element type                                                    | elementType   | "2D Thick Beam" |
| Reinforcement                                                   | reinforcement | None            |

**Assignment to Lines:**

1

## APPENDIX 2

### Attribute: 1 Title: Steel

Sub Type = Isotropic Material

Assigned in: Analysis 1

| Property        | Symbol | Value         |
|-----------------|--------|---------------|
| Young's modulus | E      | 189000000,000 |
| Poisson's ratio | nu     | 0,300         |
| Density         | rho    | 0,000         |

Loadcase ID: 2 Title: Loadcase 1

Assignment to Lines:

1

### Attribute: 2 Title: Joint material: Soil

Sub Type = Joint Material, Elasto-Plastic (Tension and Compression Equal)

Assigned in: Analysis 1

|                              | u            | v                                                     |
|------------------------------|--------------|-------------------------------------------------------|
| Number of degrees of freedom | nDOF         | 2                                                     |
| Joint type                   | JointType    | "Plane stress / plane strain / 2D axisymmetric membra |
| Assignment type              | Assignment   | "Line"                                                |
| Yield force                  | F[0]         | 0,000                                                 |
| Yield force                  | F[1]         | F                                                     |
| Hardening stiffness          | H[0]         | 0,000                                                 |
| Hardening stiffness          | H[1]         | 10,000                                                |
| Elastic spring stiffness     | K[0]         | 0,000                                                 |
| Elastic spring stiffness     | K[1]         | E                                                     |
| Mass                         | M[0]         | 0,000                                                 |
| Mass                         | M[1]         | 0,000                                                 |
| Mass position                | MassPosition | "Between nodes"                                       |

Loadcase ID: 2 Title: Loadcase 1

Assignment to Lines:

1

### Attribute: 1 Title: E

Sub Type = Interpolation Variation

| Property            | Symbol     | Value    |
|---------------------|------------|----------|
| Interpolation order | type       | "Linear" |
| Distance type       | distType   | "Actual" |
| Distance            | distance   | 0,000    |
| Distance            | distance   | 1,700    |
| Distance            | distance   | 1,710    |
| Distance            | distance   | 4,000    |
| Distance            | distance   | 4,200    |
| Distance            | distance   | 6,700    |
| Distance            | distance   | 6,710    |
| Distance            | distance   | 10,700   |
| Distance            | distance   | 14,700   |
| Distance            | distance   | 14,710   |
| Distance            | distance   | 16,710   |
| Function            | expression | "0.0"    |
| Function            | expression | "2.4E3"  |
| Function            | expression | "4.5E3"  |
| Function            | expression | "18.0E3" |
| Function            | expression | "18.0E3" |
| Function            | expression | "18.0E3" |
| Function            | expression | "18.0E3" |
| Function            | expression | "18.0E3" |
| Function            | expression | "30.0E3" |
| Function            | expression | "30.0E3" |

**Attribute: 4 Title: F**

Sub Type = Interpolation Variation

| Property            | Symbol     | Value     |
|---------------------|------------|-----------|
| Interpolation order | type       | "Linear"  |
| Distance type       | distType   | "Actual"  |
| Distance            | distance   | 0,000     |
| Distance            | distance   | 1,700     |
| Distance            | distance   | 1,710     |
| Distance            | distance   | 4,200     |
| Distance            | distance   | 6,700     |
| Distance            | distance   | 6,710     |
| Distance            | distance   | 10,700    |
| Distance            | distance   | 14,700    |
| Distance            | distance   | 14,710    |
| Distance            | distance   | 16,710    |
| Function            | expression | "0.0"     |
| Function            | expression | "43.0"    |
| Function            | expression | "47.0"    |
| Function            | expression | "136.0"   |
| Function            | expression | "226.0"   |
| Function            | expression | "221.0"   |
| Function            | expression | "362.0"   |
| Function            | expression | "502.0"   |
| Function            | expression | "899.0"   |
| Function            | expression | "1.025E3" |

## APPENDIX 2

**Attribute: 1    Title: Pinned**

**Sub Type = Structural Support**

**Assigned in: Analysis 1**

| Property         | Symbol | Value |
|------------------|--------|-------|
| Translation in X | U      | "R"   |
| Translation in Y | V      | "R"   |
| Rotation about Z | THZ    | "F"   |
| Pore pressure    | pore   | "C"   |

**Loadcase ID: 2    Title: Loadcase 1**

**Assignment to Points:**

1

**Attribute: 3 Title: Lager krafter****Sub Type = Concentrated Load**

| Property                   | Symbol       | Value    |
|----------------------------|--------------|----------|
| Concentrated load in X Dir | PX           | 2940,000 |
| Concentrated load in Y Dir | PY           | 33,000   |
| Concentrated load in Z Dir | PZ           | 0,000    |
| Moment about X axis        | MX           | 0,000    |
| Moment about Y axis        | MY           | 0,000    |
| Moment about Z axis        | MZ           | -97,000  |
| Moment about hinge nodes   | L2           | 0,000    |
| Pore pressure flux         | PorePressure | 0,000    |
| Bi moment                  | Mb           | 0,000    |

**Loadcase ID: 2 Title: Loadcase 1 Factor = 1.0****Assignment to Points:**

7

**Attribute: 9 Title: N = 2940 kN****Sub Type = Concentrated Load**

| Property                   | Symbol       | Value    |
|----------------------------|--------------|----------|
| Concentrated load in X Dir | PX           | 2940,000 |
| Concentrated load in Y Dir | PY           | 0,000    |
| Concentrated load in Z Dir | PZ           | 0,000    |
| Moment about X axis        | MX           | 0,000    |
| Moment about Y axis        | MY           | 0,000    |
| Moment about Z axis        | MZ           | 0,000    |
| Moment about hinge nodes   | L2           | 0,000    |
| Pore pressure flux         | PorePressure | 0,000    |
| Bi moment                  | Mb           | 0,000    |

**Loadcase ID: 3 Title: Loadcase 2 Factor = 1.0****Assignment to Points:**

7

**Attribute: 1    Title: Joint coordinate system**

Sub Type = Local Coordinates

| Property       | Symbol       | Value       |
|----------------|--------------|-------------|
| Attribute type | type         | "Cartesian" |
| Rotation angle | angle        | 270,000     |
| Origin         | originPos[0] | 0,000       |
| Origin         | originPos[1] | 0,000       |
| Origin         | originPos[2] | 0,000       |
| Type           | plane        | "XY_Angle"  |

Assignment to Points:

7

**Loadcases**

| Loadcase | Gravity | Title      | Analysis   | Type |
|----------|---------|------------|------------|------|
| 2        |         | Loadcase 1 | Analysis 1 |      |
| 3        |         | Loadcase 2 | Analysis 2 |      |

Gravity (when chosen) is applied as a body force of  $-9,81000\text{m/s}^2$  in the Y direction