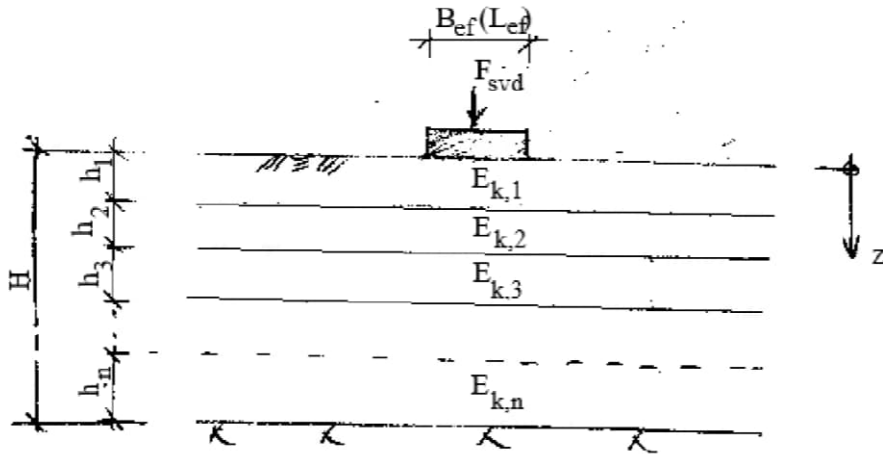


Object: Support 1 - SLS

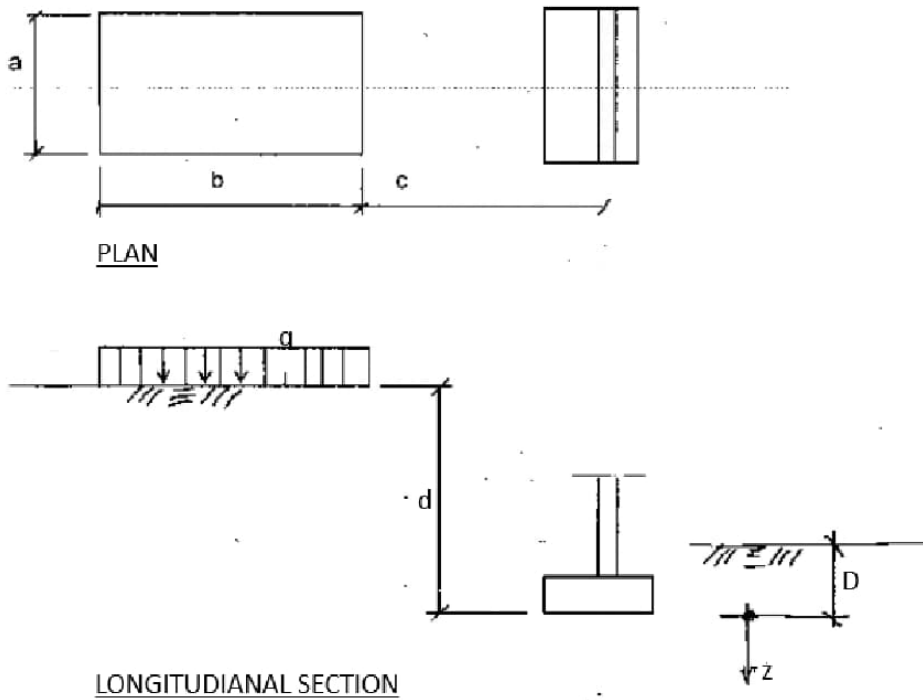
PRINCIPLE SKETCH

Calculation accordint to TRVINFRA-00227 attachment 4.

Defintion soil layers



Effect load from nearby bank



INPUT**Vertical design load**

$F_{Svd} := 4711 \cdot kN$

$B_{ef} := 3.52 \cdot m$

$L_{ef} := 8.0 \cdot m$

Vertical pressure at foundation level

(Reduction considers long-time loading before production of support)

$\sigma_{vo} := 0 \cdot kPa$

Technical lifespan

$t := 120 \cdot years$

Existance of "silt"

Dominates "silt" ("ja" or "nej") :

$silt := \text{"Nej"}$

Foundation depth

$D := 1.8 \cdot m$

Additional pressure due to load on connecting bank

Long-term load from connecting bank that acts after production of support. Normally no live load ist consider for SLS-Q noll.

$a := 8.0 \cdot m$

$b := 2 \cdot a = 16 \cdot m$

$c := 1.1 \cdot m$

$d := 6.0 \cdot m$

$q := 0 \cdot kPa$

Defintion soil layers

Number of layers (minimum of 2 pcs):

$n := 3 \cdot pcs$

Earth-factor λ is defined by soil composition: $\lambda = 0$ for coarse frictional material $\lambda = 0.5$ for "silt" $\lambda = 1.0$ for cohesion materialPressure exponent β : $\beta = 1.0$ for overconsolidated cohesion material $\beta = 0.5$ for coarse "silt", sand and gravel

Layer	E_d	E_k	h	γ	λ	β
1	50	35	2,00	18	0	0,5
2	50	35	2,00	18	0	0,5
3	50	35	2,00	18	0	0,5
-	MPa	MPa	m	kN/m ³	-	-

CALCULATIONS**Effect of "silt"**

$$\gamma_{Rd} := \text{if}(\text{silt} = \text{"Ja"}, 1.1, 1.0)$$

$$\gamma_{Rd} = 1.0$$

Total layer thickness

$$H := \sum_{i=1}^n h_i$$

$$H = 6.000 \text{ m}$$

Levels of studied soil layers

$$z_s = \begin{cases} z_1 = 0 \text{ mm} \\ \text{for } i \in 1..n \\ \begin{cases} z_{2,i} = z_{2,i-1} + h_i - 1 \text{ mm} \\ z_{2,i+1} = z_{2,i} + 1 \text{ mm} \end{cases} \end{cases}$$

z_s [m]	0	2,000	2,001	4,000	4,001	6,000	6,001
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Function - settlement modulus

Characteristic values:

$$E_{sk} = \begin{cases} E_{sk_1} = E_{k_1} \\ \text{for } i \in 1..n-1 \\ \begin{cases} E_{sk_{2,i}} = E_{k_i} \\ E_{sk_{2,i+1}} = E_{k_{i+1}} \end{cases} \\ E_{sk_{2,n}} = E_{k_n} \\ E_{sk_{2,n+1}} = 1000 \text{ MPa} \end{cases}$$

E_{sk} [MPa]	35	35	35	35	35	35	1000
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$$E_{sk}(z) := \text{linterp}(z_s, E_{sk}, z)$$

Design settlement values :

$$E_{sd} = \begin{cases} E_{sd_1} = E_{d_1} \\ \text{for } i \in 1..n-1 \\ \begin{cases} E_{sd_{2,i}} = E_{d_i} \\ E_{sd_{2,i+1}} = E_{d_{i+1}} \end{cases} \\ E_{sd_{2,n}} = E_{d_n} \\ E_{sd_{2,n+1}} = 1000 \text{ MPa} \end{cases}$$

$E_{sd} [\text{MPa}]$	50	50	50	50	50	50	1000
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$$E_{sd}(z) := \text{linterp}(z_s, E_{sd}, z)$$

Function - earth factor

$$\lambda_s = \begin{cases} \lambda_{s_1} = \lambda_1 \\ \text{for } i \in 1..n-1 \\ \begin{cases} \lambda_{s_{2,i}} = \lambda_i \\ \lambda_{s_{2,i+1}} = \lambda_{i+1} \end{cases} \\ \lambda_{s_{2,n}} = \lambda_n \\ \lambda_{s_{2,n+1}} = 0 \end{cases}$$

$\lambda_s [-]$	0	0	0	0	0	0	0
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$$\lambda(z) = \text{linterp}(z_s, \lambda_s, z)$$

Function - density

$$\gamma_s = \begin{cases} \gamma_{s_1} = \gamma_1 \\ \text{for } i \in 1..n-1 \\ \begin{cases} \gamma_{s_{2,i}} = \gamma_i \\ \gamma_{s_{2,i+1}} = \gamma_{i+1} \end{cases} \\ \gamma_{s_{2,n}} = \gamma_n \\ \gamma_{s_{2,n+1}} = 0 \end{cases}$$

$\gamma_s [\text{kN/m}^3]$	18	18	18	18	18	18	0
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$$\gamma(z) = \text{linterp}(z_s, \gamma_s, z)$$

Function - pressure exponent

$$\beta_s = \begin{cases} \beta_{s_1} = \beta_1 \\ \text{for } i \in 1..n-1 \\ \left| \begin{array}{l} \beta_{s_{2,i}} = \beta_i \\ \beta_{s_{2,i+1}} = \beta_{i+1} \end{array} \right. \\ \beta_{s_{2,n}} = \beta_n \\ \beta_{s_{2,n+1}} = 0 \end{cases}$$

$$\beta(z) = \text{linterp}(z_s, \beta_s, z)$$

Effective loading

$$q_{netto} := \frac{F_{Svd}}{B_{ef} \cdot L_{ef}} - \sigma_{vo}$$

$$q_{netto} = 167 \text{ kPa}$$

Additional pressure due to load on connecting bank

$$m = \frac{a}{2(z+d)}$$

$$n_1 = \frac{b+c}{d+z}$$

$$n_2 = \frac{c}{d+z}$$

$$I_1 = \frac{1}{2\pi} \left[\frac{m \cdot n_1 \cdot (2 + m^2 + n_1^2)}{(1+m^2) \cdot (1+n_1^2) \cdot \sqrt{1+m^2+n_1^2}} + \text{atan} \left(\frac{m \cdot n_1}{\sqrt{1+m^2+n_1^2}} \right) \right]$$

$$I_2 = \frac{1}{2\pi} \left[\frac{m \cdot n_2 \cdot (2 + m^2 + n_2^2)}{(1+m^2) \cdot (1+n_2^2) \cdot \sqrt{1+m^2+n_2^2}} + \text{atan} \left(\frac{m \cdot n_2}{\sqrt{1+m^2+n_2^2}} \right) \right]$$

$$\sigma'_{\text{tillskott}} = 2 \cdot q \cdot (I_1 - I_2)$$

Time factor

$$\chi_{tid} := 1 + 0.2 \cdot \log(10 \cdot t)$$

$$\chi_{tid} = 1.62$$

Settlement method 1

$$\Delta\sigma_v = q_{\text{netto}} \frac{B_{\text{ef}} \cdot L_{\text{ef}}}{(B_{\text{ef}} + z)(L_{\text{ef}} + z)} + \sigma'_{\text{tillskott}}$$

$$\eta := 0.70$$

Charateristic settlement:

$$s_{k,l} := \eta \cdot \chi_{\text{tid}} \cdot \int_{0 \cdot m}^H \frac{\Delta\sigma_v(z)}{E_{\text{kar}}(z)} dz \quad s_{k,l} = 14.8 \text{ mm}$$

Design settlement:

$$s_{d,l} := \eta \cdot \chi_{\text{tid}} \cdot \int_{0 \cdot m}^H \frac{\Delta\sigma_v(z)}{E_{\text{dim}}(z)} dz \quad s_{d,l} = 10.4 \text{ mm}$$

Settlement method 2

$$g = 1 + 21.5 \cdot \left(\frac{B_{ef}}{L_{ef}} + 2.5 \right)^{-2.15}$$

$$g = 3.12$$

$$\sigma_{v,mo} = \sigma_{vo} + \gamma(z) \cdot z$$

$$\Delta\sigma_v = \begin{cases} \sigma'_{tillskott} & \text{if } \left(1 - \frac{1}{g} \cdot \frac{z}{B_{ef}} \right) \leq 0 \\ q_{netto} \left[1 + (3 - 2 \cdot \lambda(z)) \cdot \frac{1}{g} \cdot \frac{z}{B_{ef}} \right] \cdot \left(1 - \frac{1}{g} \cdot \frac{z}{B_{ef}} \right)^3 + \sigma'_{tillskott} & \text{if } \left(1 - \frac{1}{g} \cdot \frac{z}{B_{ef}} \right) > 0 \end{cases}$$

$$P_a := 100 \cdot kPa$$

$$\eta := 0.65$$

Charateristic settlement:

$$\varepsilon(z) = \frac{P_a}{E_{sk} \beta(z)} \left[\left(\frac{\sigma_{v,mo} + \Delta\sigma_v}{P_a} \right)^{\beta(z)} - \left(\frac{\sigma_{v,mo}}{P_a} \right)^{\beta(z)} \right]$$

$$s_{k,2} := \eta \cdot \chi_{tid} \cdot \int_{0 \cdot m}^H \varepsilon(z) dz$$

$$s_{k,2} = 21.6 \text{ mm}$$

Design settlement:

$$\varepsilon(z) = \frac{P_a}{E_{sd} \beta(z)} \left[\left(\frac{\sigma_{v,mo} + \Delta\sigma_v}{P_a} \right)^{\beta(z)} - \left(\frac{\sigma_{v,mo}}{P_a} \right)^{\beta(z)} \right]$$

$$s_{d,2} := \eta \cdot \chi_{tid} \cdot \int_{0 \cdot m}^H \varepsilon(z) dz$$

$$s_{d,2} = 15.1 \text{ mm}$$

Settlement method 3

$$r_o := \sqrt{\frac{L_{ef} \cdot B_{ef}}{\pi}}$$

$$r_o = 2.99 \text{ m}$$

$$\Delta S = \begin{cases} \text{for } i \in 1..n \\ \quad n_{del} = \text{trunc}\left(\frac{h_i}{0.1\text{m}}\right) \\ \quad \Delta h = \frac{h_i}{n_{del}} \\ \quad \Delta S_i = 0 \\ \quad \text{for } j \in 1..n_{del} \\ \quad \Delta S_i = \Delta S_i + 3.87 \cdot \left[\frac{z_{s2,i-1} + \Delta h \cdot (j-1)}{r_o} + 1.82 \right]^{-1.7} - 3.87 \cdot \left(\frac{z_{s2,i-1} + \Delta h \cdot j}{r_o} + 1.82 \right)^{-1.7} \end{cases}$$

$\Delta S [-]$	0,58	0,27	0,15	0,50	0,50	0,50	0
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$$r_l := 0.5 \cdot m$$

$$c := \frac{4 \cdot r_l \cdot r_o}{(r_l + r_o)^2}$$

$$c = 0.49$$

Verification of conditon r_e :

$$r_e := \begin{cases} \text{if } 1 \leq \frac{L_{ef}}{B_{ef}} \leq 20 \\ \quad \left\| \left\| 0.45 + 0.98 \cdot \left(\frac{L_{ef}}{B_{ef}} + 2.0 \right)^{-0.42} \right. \right. \\ \quad \text{else} \\ \quad \left\| \left\| \text{"Avbryt"} \right. \right. \end{cases}$$

$$r_e = 0.983$$

$$d_e := 0.82 + 0.96 \cdot \left(\frac{D}{r_o} + 2.0 \right)^{-2.4}$$

$$d_e = 0.92$$

$$\eta := 1.10$$

Charateristic settlement:

$$s_o := q_{netto} \cdot r_o \cdot \sum_{i=1}^n \frac{\Delta S_i}{E_{k_i}} \quad s_o = 14.3 \text{ mm}$$

$$s_{k,3} := \eta \cdot \chi_{tid} \cdot \left(c \cdot r_e \cdot d_e \cdot s_o + \int_{0 \cdot m}^H \frac{\sigma'_{tillskott}(z)}{E_{kar}(z)} dz \right) \quad s_{k,3} = 11.3 \text{ mm}$$

Design settlement:

$$s_o := q_{netto} \cdot r_o \cdot \sum_{i=1}^n \frac{\Delta S_i}{E_{d_i}} \quad s_o = 10.0 \text{ mm}$$

$$s_{d,3} := \eta \cdot \chi_{tid} \cdot \left(c \cdot r_e \cdot d_e \cdot s_o + \int_{0 \cdot m}^H \frac{\sigma'_{tillskott}(z)}{E_{dim}(z)} dz \right) \quad s_{d,3} = 7.9 \text{ mm}$$

RESULTS**Characteristic settlement**

$$s_k := \frac{s_{k,1} + s_{k,2} + s_{k,3}}{3} \quad s_k = 15.9 \text{ mm}$$

Design settlement

$$s_d := \frac{s_{d,1} + s_{d,2} + s_{d,3}}{3} \quad s_d = 11.1 \text{ mm}$$